

POLLUTION ABATEMENT SERVICES
IMPROVEMENT SUGGESTED FOR IMMEDIATE CONSTRUCTION
(CONTRACT DOCUMENTS TO BE PREPARED
BY URS CONSULTANT UNDER TASK-5
OF O&M CONTRACT)

Note: Meeting with TH-AL & Sean
Encl 2/24/91
All agreed except
item B(1).
Also wait
until EPA
decide on
treatment
plant.
AKG

(A) LEACHATE COLLECTION TANK:

1. Piping modifications to replace existing ball valve and prevent back flow to wells.
2. Seal hairline cracks and provide suitable seal coat on inside of tank.
3. Insulate roof - add couple vents.
4. Rebuild end walls (top of concrete tank and below roof) and install double pane glass doors.
5. Insulate around tank up to top of HDPE liner.
6. Install explosion proof lighting inside tank.
7. Install flood light outside tank.
8. Install level control instrumentation with heat tracer to eliminate freezing and connect system to automatic dialer.
9. Install independent pumping system in leachate collection tank with metering arrangement and easy non-drip tanker hook-up for easy loading of leachate.
- 10.* Install leak proof liner inside tank.
- 11.* Provide additional heat with thermostat control to eliminate freezing.

(B) DECON PAD & ACCESS ROAD MODIFICATIONS:

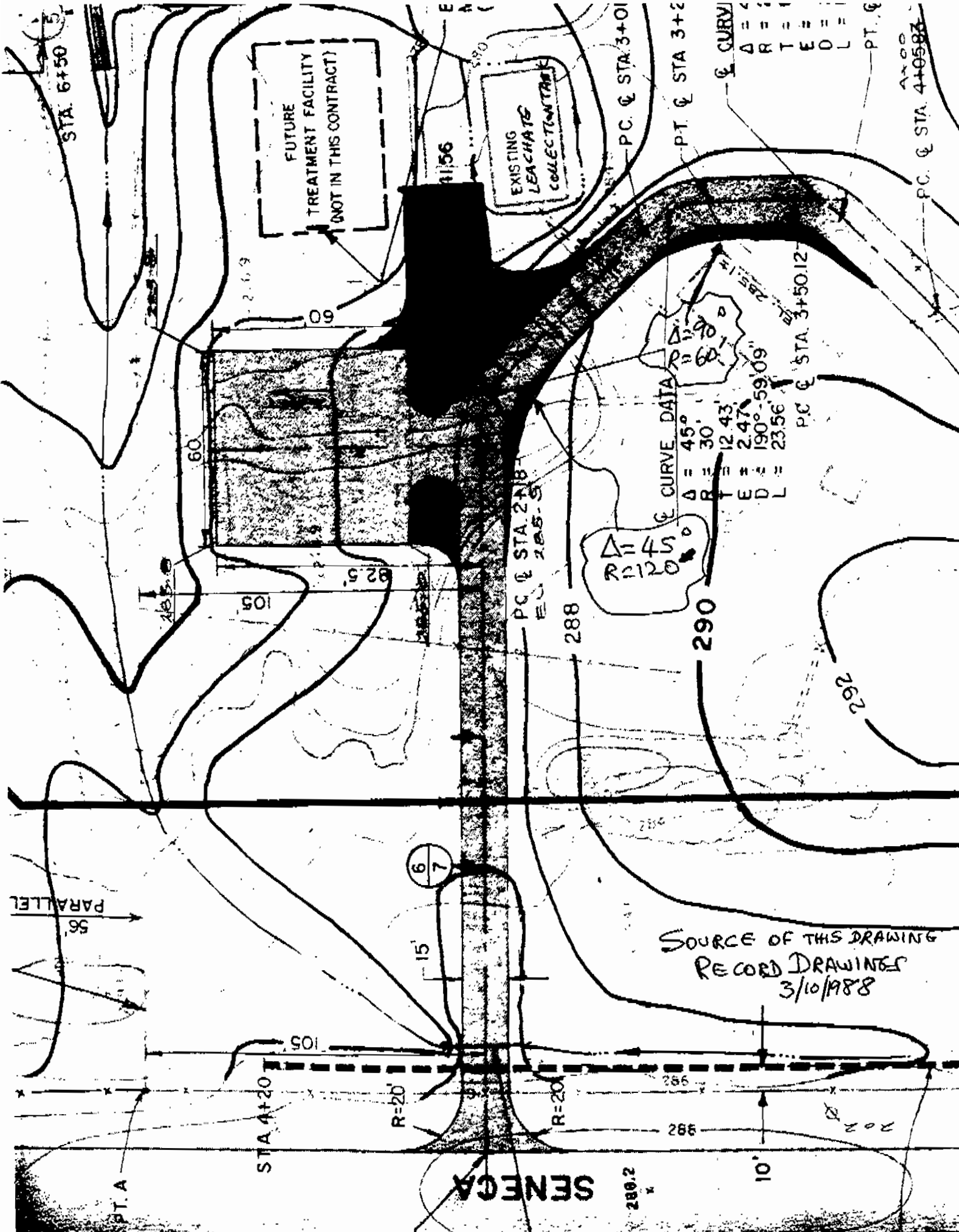
1. Construct 50' x 20' ^{contained} decon and leachate loading platform as shown on attached drawing.
2. Modify access road and parking area as shown.

NO by AKG

(C) ELECTRICAL SYSTEM:

1. Electrical line from main control panel to leachate tank for pump installation.
2. Strip pad heater's inside all control panels and automatic dialer.

*NOTE: Need to be evaluated this winter



SOURCE OF THIS DRAWING
 RECORD DRAWINGS
 3/10/1988

AK

New York State Department of Environmental Conservation
50 Wolf Road, Albany, New York 12233



Thomas C. Jorling
Commissioner

Mr. Richard Ramon, P.E.
Emergency and Remedial Response Division
U.S. Environmental Protection Agency
Region II
26 Federal Plaza - Room 29-100
New York, NY 10278

JUN 21 1991

RE: Pollution Abatement Services (PAS)
Oswego County, Site #7-38-001

Dear Mr. Ramon:

Enclosed for your information is a copy of the Final Report for Leachate Collection System and Cap Evaluation, as it relates to the Long-term Operation and Maintenance of the PAS site. This report was prepared by the URS Consultants, Inc. (NYSDEC Consultant) as part of their O&M task assignment.

If you have any questions, please call me at 518/457-0927.

Sincerely,

A. K. Gupta, P.E.
Environmental Engineer 2
Operation & Maintenance Section
Bureau of Construction Services
Division of Hazardous Waste Remediation

Enclosure

bcc: G. Rider
R. Lupe

a:ramon.pas:AKG:et



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3011

The first of these is the fact that the law of international law is not a single, unified system. It is a complex of many different legal regimes, each with its own set of rules and principles. This complexity is reflected in the fact that there is no single international court or tribunal that has jurisdiction over all international law. Instead, there are many different courts and tribunals, each with its own jurisdiction and powers. This complexity makes it difficult to understand and apply international law, and it is one of the main reasons why international law is often described as a "jumble of legal regimes".

The second of these factors is the fact that international law is not a static system. It is constantly evolving and changing, as new legal regimes are created and old ones are modified or abolished. This evolution is driven by a number of factors, including changes in the international system, changes in the interests of states, and changes in the values and norms of the international community. This evolution makes it difficult to understand and apply international law, and it is one of the main reasons why international law is often described as a "living system".

The third of these factors is the fact that international law is not a self-contained system. It is closely linked to the law of the individual states, and it often draws on the principles and rules of domestic law. This linkage makes it difficult to understand and apply international law, and it is one of the main reasons why international law is often described as a "branch of law".

The fourth of these factors is the fact that international law is not a uniform system. It varies from state to state, and it is often subject to different interpretations and applications. This variation makes it difficult to understand and apply international law, and it is one of the main reasons why international law is often described as a "pluralist system".

The fifth of these factors is the fact that international law is not a self-enforcing system. It lacks a central authority that can enforce its rules and principles. This lack of enforcement makes it difficult to understand and apply international law, and it is one of the main reasons why international law is often described as a "voluntary system".

The sixth of these factors is the fact that international law is not a self-protecting system. It is often subject to external pressures and influences, which can lead to changes in its rules and principles. This external influence makes it difficult to understand and apply international law, and it is one of the main reasons why international law is often described as a "dynamic system".

The seventh of these factors is the fact that international law is not a self-defining system. It is often subject to external definitions and interpretations, which can lead to changes in its rules and principles. This external definition makes it difficult to understand and apply international law, and it is one of the main reasons why international law is often described as a "flexible system".

The eighth of these factors is the fact that international law is not a self-sustaining system. It is often subject to external support and assistance, which can lead to changes in its rules and principles. This external support makes it difficult to understand and apply international law, and it is one of the main reasons why international law is often described as a "dependent system".

The ninth of these factors is the fact that international law is not a self-renewing system. It is often subject to external renewal and revitalization, which can lead to changes in its rules and principles. This external renewal makes it difficult to understand and apply international law, and it is one of the main reasons why international law is often described as a "renewable system".

The tenth of these factors is the fact that international law is not a self-terminating system. It is often subject to external termination and abolition, which can lead to changes in its rules and principles. This external termination makes it difficult to understand and apply international law, and it is one of the main reasons why international law is often described as a "terminable system".

SUBJECT: [Illegible]

[Illegible typed text]

REFERENCE: [Illegible]

[Illegible typed text]

[Illegible typed text]

RECOMMENDATION:

[Illegible typed text]

[Illegible typed text]

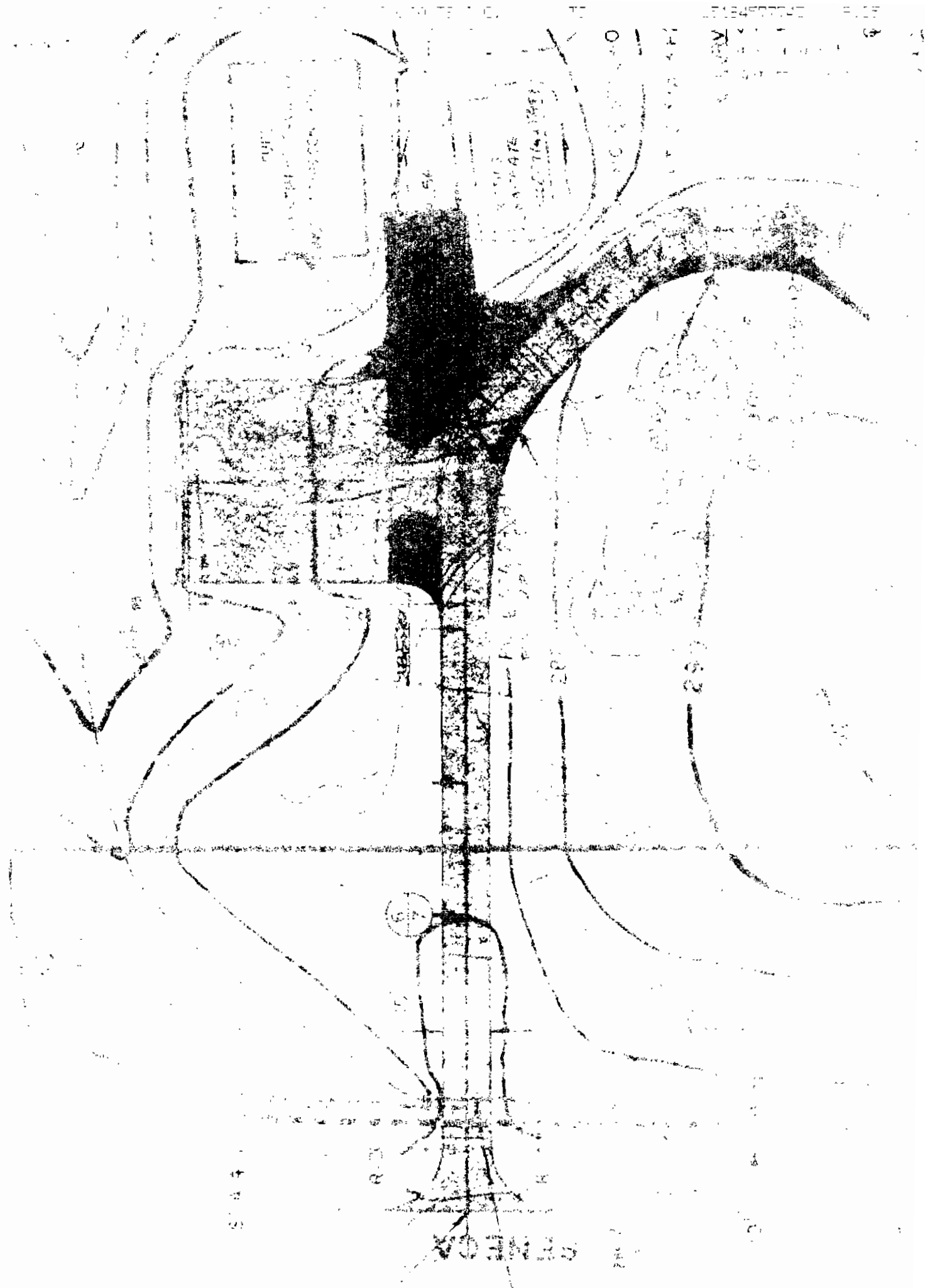
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TABLE 1

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Item	Description	Value
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URS CONSULTANTS, INC.

URS CONSULTANTS, INC

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July 10, 1991

Mr. Robert Gupta, Project Manager
URS Consultants, Inc.
Department of Hazardous Waste Remediation
New York Department of Environmental Conservation
100 Wolf Road
Plainville, New York 12233-7010

Re: The SITE (aka SITE) NO. 8-001 (W.A. 84034-1-8)

Dear Mr. Gupta:

As per your request of last week, we have developed preliminary remedial
investigation (PRI) data for the site. The data includes a list of
potential contaminants, a list of potential receptors, and a list of
potential receptors. The data is provided on the attached sheets.

If you have any questions, please do not hesitate to call me.

Very truly yours,

URS CONSULTANTS, INC.

URS CONSULTANTS, INC. PO BOX
ANN ARBOR, NEW YORK

Post-it Transfer

To: A. K. Gupta
On: 11/10/91
From: DJWR - URS
Fax: 855-444-4444

11/10/91
URS
855-444-4444

DJWR

11/10/91 11:21

cc:

- 1. Bolton - URS
- 2. Hoffman - URS
- 3. L. L. - URS
- 4. 35236 00 (1995)

TABLE 1

EAS SLOP O&M
 IMPROVEMENTS SUGGESTED FOR OPERATED SYSTEMS
 PRELIMINARY COST ESTIMATES

1. WATER COLLECTION TANK

1	2.5 Ton Weight Capacity	\$ 1,100
	2-1/2" Ball Valve Closure	\$12,000
	Insulate Roof	\$10,000
2	Retain Lid Walls	\$ 3,000
3	Lightning Protection with 100' to 100' Leads	\$ 2,000
4	Light Inside Tank	\$ 5,000
5	Outside Floor Light	\$ 1,000
6	Level Control Instrumentation	\$ 1,000
7	Leachate Pumping system in Tank	\$12,000
8	HDPE Liner Inside Tank	\$12,000
9	Additional Heat	\$10,000

2. LEACHATE COLLECTION NOODLES

1	Loading Platform	\$ 4,000
2	Slider Road	\$ 2,000

ELECTRICAL SYSTEM

1	Line from Main Panel to Pump	\$ 1,000
2	Strip Pad Heater	\$ 1,000
3	AMM meters for Well Pumps	\$1,000

TOTAL \$27,000

NOTE: Costs for work listed and provided are based on 1980 contract prices.

TABLE 2

PHO SITE O&M
 INITIAL O&M VIS - SCOPE OF THE IMMEDIATE CONSTRUCTION
 BASIS FOR COST ESTIMATES

PHYSICAL WORK NECESSARY

1. Piping Modifications - Modify piping as indicated
2. Seal Pipeline Cracks - Seal and clean tank. Ins. cracks, provide access opening to seal all cracks. Apply epoxy sealer to interior walls.
3. Insulate Roof - Apply 4" fibreglass insulation, covered with 2" of install - edge and end cap tin hang, and provide 2" thermal break. Add edge-walk and eye ladders. (NOTE: existing roof structure may have to be removed for work inside tank).
4. Rebuild end walls - Rebuild existing end walls with 12" thick weather tight insulated walls (preserved end steel in insulation with stake stringers). Add new steel stringers and 12" x 12" suspension (with 2" edge size for height) floor to provide for easier equipment access and egress from tank.
5. Install inside wall to back lower wall - Install 12" thick wall of type to HDPE liner and install 2" slope floor to interior floor. To top of wall - add floating foam insulation and top of wall to be water tight.
6. Light Inside tank - Add 4 each 115 watt NF-A lights, including conduit and wiring.
7. Outside Floor Light - Add 2 each 250 watt flood lights, including conduit and wiring.
8. Level Control Insulation - Install 2" of insulation, conductive thermal barrier, multiple set points, with 2" insulation. Heat tracing not required. 2nd unit as for tank-1.
9. Install Leachate Pumping System in Tank - Install 1" diameter pump and entire discharge piping with 1" diameter pipe to tank discharge in 2 lockers for handling. HDPE liner to be installed in discharge pipe, with 2" insulation. 2" diameter pipe to be installed in 2 lockers.
10. HDPE liner inside Tank - Install 12" thick HDPE liner inside tank.

TABLE 2 (cont'd)

admission. When installed, use an 4' x 20' sheet of plastic to cover the 4' x 20' area, leaving an edge of 6" over the edge of the plastic. This will prevent the plastic from being pulled away by the wind. The plastic should be secured to the ground by using heavy stones or bricks. The plastic should be replaced as soon as it becomes dirty or damaged.

4. WATER COLLECTION SYSTEMS

1. Loading Platform - Remove top of 2' x 4' x 4' platform, to form collection tank and fill.

Widen road - Fill road to level of water level. Return.

5. WATER STORAGE SYSTEMS

1. Use Iron Main - Run 1" pipe at 10' level. Use 1" x 1" x 1" panel board and breaker.

2. Grid Feeder - Use 1" x 1" x 1" panel board and breaker.

Equipment for well pumps - In 20' diameter, 10' x 10' x 10' collection well pump, use transfer pump in tank.

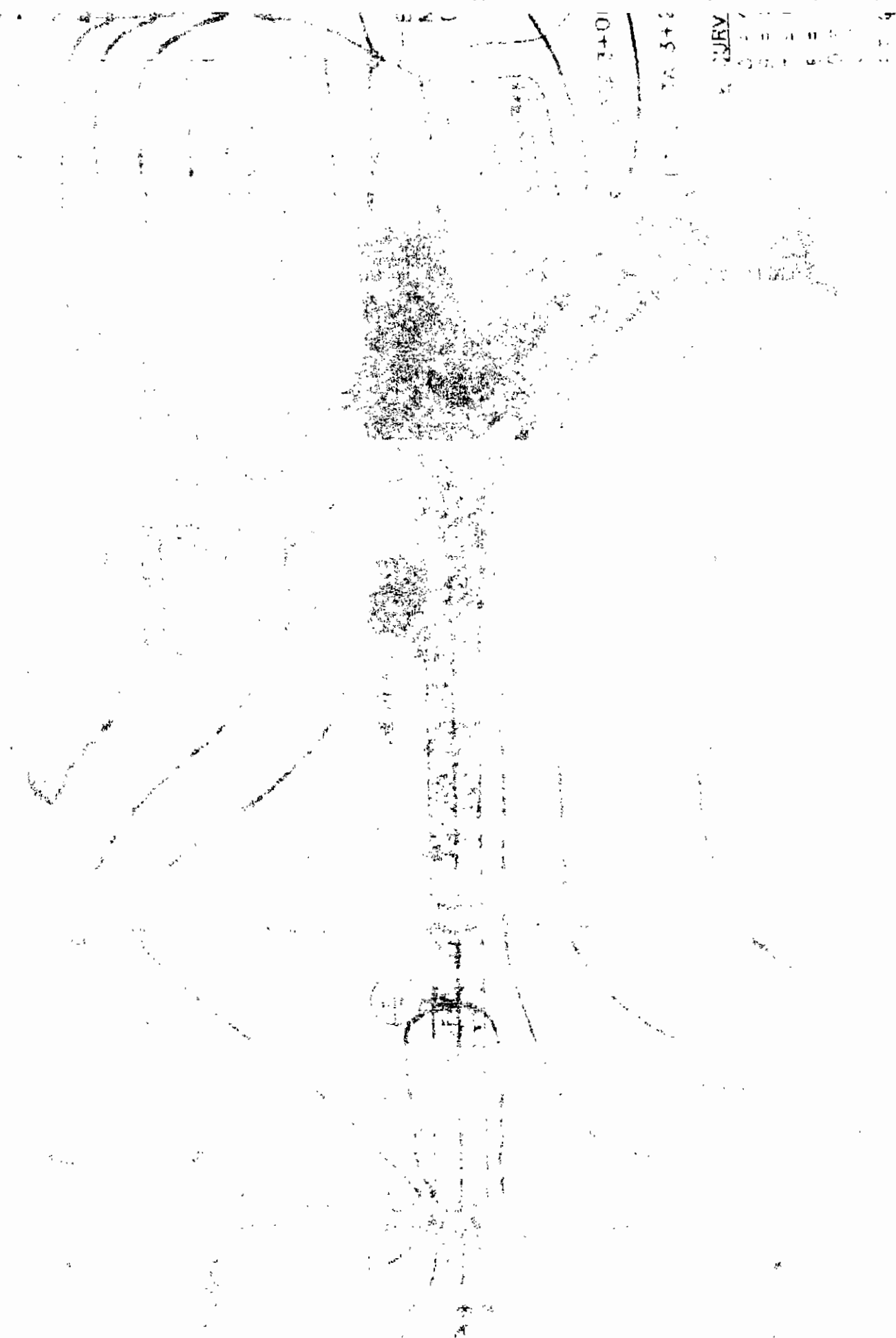
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JURY

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100





AN INTERNATIONAL PROFESSIONAL SERVICES ORGANIZATION

URS CONSULTANTS, INC.
282 DELAWARE AVENUE
BUFFALO, NEW YORK 14202-1801
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SEATTLE
VIRGINIA BEACH
WASHINGTON, DC

discussed with Jerry
Hold for PRP response

Jerry
Construction cost estimate at the suggested improvements seems to be OK. The estimate we should go for it.
Thanks!
P.S. What about Debra Pad? I think this is needed.
AK
7/10

July 10, 1991

Mr. A.K. Gupta, P.E., Project Manager
Bureau of Construction Services
Division of Hazardous Waste Remediation
NYS Department of Environmental Conservation
50 Wolf Road
Albany, New York 12233-7010

RE: PAS SITE O&M SITE NO. 7-38-001 (W.A. D002340-8)

Dear Mr. Gupta:

As per your request last week, we have developed preliminary construction cost estimates for individual items in your fax transmittal of June 26, 1991 related to immediate improvements at the PAS site. The estimated costs of individual items and basis for the costs are provided in the attached tables.

If you have any questions, please do not hesitate to call me.

Very truly yours,

URS CONSULTANTS, INC.

Dharmarajan R. Iyer, Ph.D.
Task Manager

DI/dm
35236B/7-3-91L

Enc.

- cc: P. David Smith - NYSDEC
- J. Gorton - URS
- R. Hoffman, URS
- J. Lysiak, URS
- File 35236.00 (005)

Post-It™ brand fax transmittal memo 7671		# of pages ▶ 5	
To	A.K. Gupta	From	Dharma Iyer
Co.	NYS DEC	Co.	URS
Dept.	HWR - O&M Ser.	Phone #	716-856 5636
Fax #	518-457-7743	Fax #	716-856 2545

TABLE 1

PAS SITE O&M
IMPROVEMENT SUGGESTED FOR IMMEDIATE CONSTRUCTION
PRELIMINARY COST ESTIMATES

A. LEACHATE COLLECTION TANK

1.	Piping Modifications	\$ 1,500
2.	Seal Hairline Cracks	\$15,000
3.	Insulate Roof	\$10,000
4.	Rebuild End Walls	\$ 3,000
5.	Insulate Outside Wall to HDPE Liner	\$ 3,000
6.	Light Inside Tank	\$ 5,500
7.	Outside Floor Light	\$ 1,000
8.	Level Control Instrumentation	\$ 2,000
9.	Leachate Pumping system in Tank	\$12,000
10.	HDPE Liner Inside Tank	\$15,000
11.	Additional Heat	\$10,000

B. DECON PAD AND ACCESS LOAD MODIFICATIONS

1.	Loading Platform	\$ 4,000
2.	Widen Road	\$ 2,000

C. ELECTRICAL SYSTEM

1.	Line from Main Parcel to Pump	\$ 1,500
2.	Strip Pad Heater	\$ 500
2.	APM meters for Well Pumps	\$ 1,500

TOTAL		\$87,500
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NOTE: Costs for work inside tank are based on Level 'C' Protection.

TABLE 2

PAS SITE O&M
IMPROVEMENTS SUGGESTED FOR IMMEDIATE CONSTRUCTION
BASIS FOR COST ESTIMATES

A. LEACHATE COLLECTION TANK

1. Piping Modifications: Modify piping with air-relief.
2. Seal Hairline Cracks: Drain and clean tank, clean cracks, pressure inject epoxy to seal all cracks, apply epoxy coating to interior walls.
3. Insulate Roof: Apply R5.7 fiberglass insulation over existing roof; install edge and end cap flashing, and single ply membrane roofing. Add ridge-vent and eave vents. (NOTE: existing metal deck roofing may have to be removed for work inside tank).
4. Rebuild End Walls: Remove existing endwalls and replace with new weather tight insulated walls (preserved wood sheeting, insulation and wood shake shingles). Add new steel doorways with wire glass viewpanes. (NOTE: larger size (or height) door may be needed for easier equipment access and egress from tank).
5. Insulate Outside Wall to HDPE Liner Wall: Excavate around perimeter of tank to HDPE liner and install 2" styrofoam insulation from liner to top of wall, add flashing over insulation and top of wall to make water tight.
6. Light Inside Tank: Add 4 each 175 watt XP mercury vapor lights with conduit and wiring.
7. Outside Flood Light: Add 2 each 250 watt mercury vapor lights (photo cell) with conduit and wiring.
8. Level Control Instrumentation: Install two (2) ultrasonic non-contact transceivers (multiple set points) with conduits and wiring (heat tracing not required; 2nd unit is for back-up).
9. Install Leachate Pumping System in Tank: Install leachate transfer pump and exterior discharge piping with quick couple disconnect to load leachate into tankers for hauling (NOTE: May need to evaluate submersible pump versus centrifugal split casing transfer pump).
10. HDPE Liner Inside Tank: Clean and acid wash tank, install 40 ml HDPE liner inside tank.

TABLE 2 (Cont'd)

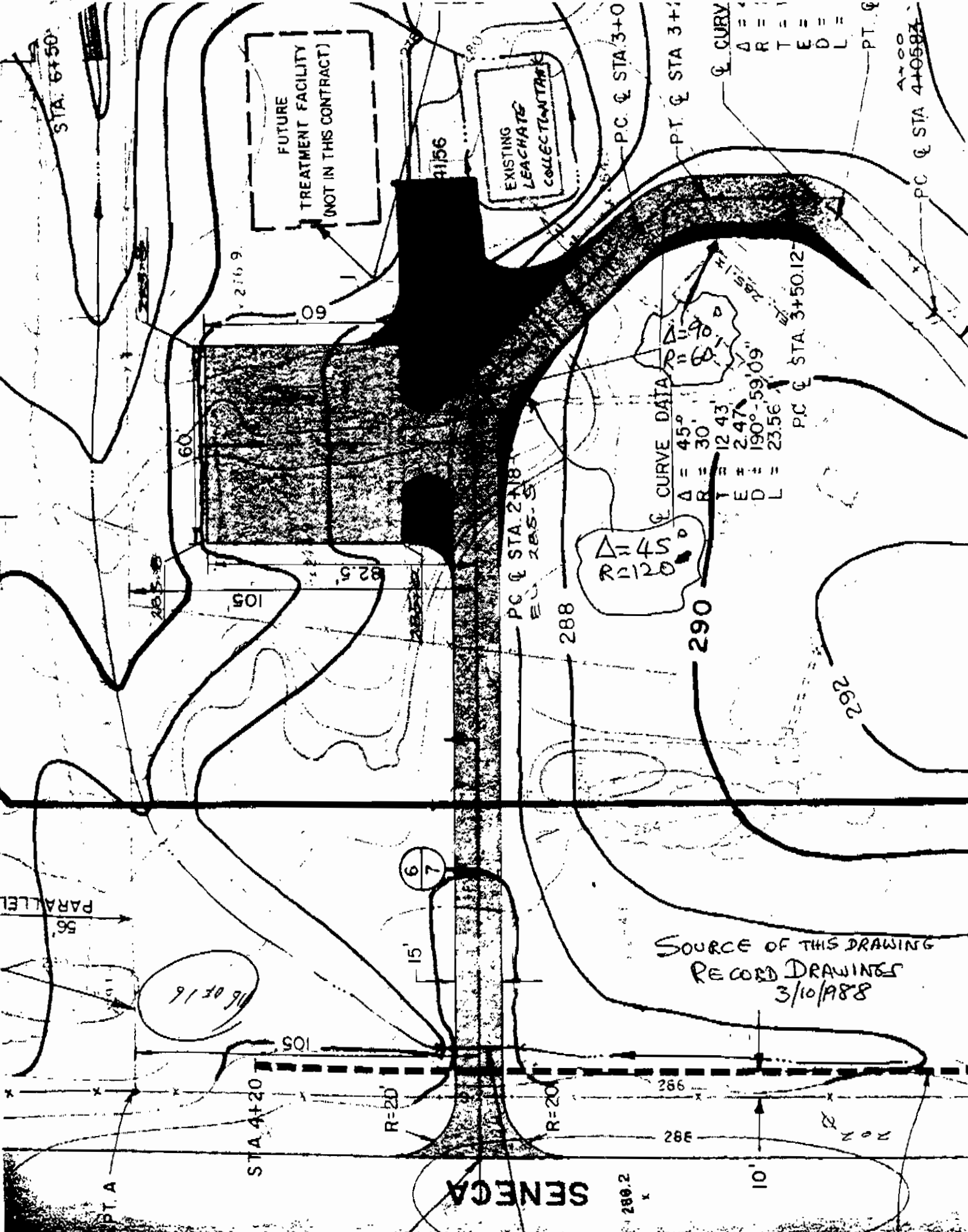
11. **Additional Heat:** Install two XP 35 MBH electric heaters with thermostats, wiring and controls (NOTE: electrical consumption may require an assessment of load availability on the incoming electrical service).

B. DECON PAD AND ACCESS

1. **Loading Platform:** Remove topsoil, build asphalt pad with drain to collection tank, and fill.
2. **Widen Road:** Widen road in areas shown on attached sketch.

C. ELECTRICAL SYSTEM

1. **Line from Main Road to Pump:** Install new service riser with line, panel board and breaker.
2. **Strip Pad Heater:** Install heater pads in all panels.
3. **Ampmeter for Well Pumps:** Install ammeters for all four leachate collection well pumps and transfer pump in tank.



SOURCE OF THIS DRAWING
 RECORD DRAWINGS
 3/10/1988

DRAFT

POLLUTION ABATEMENT SERVICES
IMPROVEMENT SUGGESTED FOR IMMEDIATE CONSTRUCTION
(CONTRACT DOCUMENTS TO BE PREPARED
BY URS CONSULTANT UNDER TASK-5
OF O&M CONTRACT)

*Requested URS
to submit Cost
of these
activities*

(A) LEACHATE COLLECTION TANK:

1. Piping modifications to replace existing ball valve and prevent back flow to wells.
2. Seal hairline cracks and provide suitable seal coat on inside of tank.
3. Insulate roof - add couple vents.
4. Rebuild end walls (top of concrete tank and below roof) and install double pane glass doors.
5. Insulate around tank up to top of HDPE liner.
6. Install explosion proof lighting inside tank.
7. Install flood light outside tank.
8. Install level control instrumentation with heat tracer to eliminate freezing and connect system to automatic dialer.
9. Install independent pumping system in leachate collection tank with metering arrangement and easy non-drip tanker hook-up for easy loading of leachate.
- 10.* Install leak proof liner inside tank.
- 11.* Provide additional heat with thermostat control to eliminate freezing.

(B) DECON PAD & ACCESS ROAD MODIFICATIONS:

1. Construct 50' x 20' ^{contained} decon and leachate loading platform as shown on attached drawing.
2. Modify access road and parking area as shown.

(C) ELECTRICAL SYSTEM:

1. Electrical line from main control panel to leachate tank for pump installation.
2. Strip pad heater's inside all control panels and automatic dialer.
3. *Install amp. meters on well pumps*

*NOTE: Need to be evaluated. *the units*

URS

INTERNATIONAL PROFESSIONAL SERVICE CORPORATION

URS CONSULTANTS, INC.

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July 10, 1991

Mr. A.K. Gupta, P.E., Project Manager
Bureau of Construction Services
Division of Hazardous Waste Remediation
NYS Department of Environmental Conservation
50 Wolf Road
Albany, New York 12233-7010

JUL 15 1991

RE: PAS SITE O&M SITE NO. 7-38-001 (W.A. D002340-8)

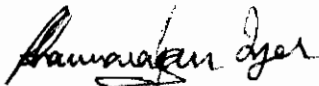
Dear Mr. Gupta:

As per your request last week, we have developed preliminary construction cost estimates for individual items in your fax transmittal of June 26, 1991 related to immediate improvements at the PAS site. The estimated costs of individual items and basis for the costs are provided in the attached tables.

If you have any questions, please do not hesitate to call me.

Very truly yours,

URS CONSULTANTS, INC.



Dharmarajan R. Iyer, Ph.D.
Task Manager

DI/dm
35236B/7-3-91L
Enc.

cc: P. David Smith - NYSDEC
J. Gorton - URS
R. Hoffman, URS
J. Lysiak, URS
File 35236.00 (005)

TABLE 1

PAS SITE O&M
IMPROVEMENT SUGGESTED FOR IMMEDIATE CONSTRUCTION
PRELIMINARY COST ESTIMATES

A. <u>LEACHATE COLLECTION TANK</u>		
1.	Piping Modifications	\$ 1,500
2.	Seal Hairline Cracks	\$15,000
3.	Insulate Roof	\$10,000
4.	Rebuild End Walls	\$ 3,000
5.	Insulate Outside Wall to HDPE Liner	\$ 3,000
6.	Light Inside Tank	\$ 5,500
7.	Outside Floor Light	\$ 1,000
8.	Level Control Instrumentation	\$ 2,000
9.	Leachate Pumping system in Tank	\$12,000
10.	HDPE Liner Inside Tank	\$15,000
11.	Additional Heat	\$10,000
B. <u>DECON PAD AND ACCESS LOAD MODIFICATIONS</u>		
1.	Loading Platform	\$ 4,000
2.	Widen Road	\$ 2,000
C. <u>ELECTRICAL SYSTEM</u>		
1.	Line from Main Parcel to Pump	\$ 1,500
2.	Strip Pad Heater	\$ 500
2.	APM meters for Well Pumps	<u>\$ 1,500</u>
TOTAL		\$87,500

NOTE: Costs for work inside tank are based on Level 'C' Protection.

TABLE 2

PAS SITE O&M
IMPROVEMENTS SUGGESTED FOR IMMEDIATE CONSTRUCTION
BASIS FOR COST ESTIMATES

A. LEACHATE COLLECTION TANK

1. Piping Modifications: Modify piping with air-relief.
2. Seal Hairline Cracks: Drain and clean tank, clean cracks, pressure inject epoxy to seal all cracks, apply epoxy coating to interior walls.
3. Insulate Roof: Apply R5.7 fiberglass insulation over existing roof; install edge and end cap flashing, and single ply membrane roofing. Add ridge-vent and eave vents. (NOTE: existing metal deck roofing may have to be removed for work inside tank).
4. Rebuild End Walls: Remove existing endwalls and replace with new weather tight insulated walls (preserved wood sheeting, insulation and wood shake shingles). Add new steel doorways with wire glass viewpanes. (NOTE: larger size (or height) door may be needed for easier equipment access and egress from tank).
5. Insulate Outside Wall to HDPE Liner Wall: Excavate around perimeter of tank to HDPE liner and install 2" styrofoam insulation from liner to top of wall, add flashing over insulation and top of wall to make water tight.
6. Light Inside Tank: Add 4 each 175 watt XP mercury vapor lights with conduit and wiring.
7. Outside Flood Light: Add 2 each 250 watt mercury vapor lights (photo cell) with conduit and wiring.
8. Level Control Instrumentation: Install two (2) ultrasonic non-contact transceivers (multiple set points) with conduits and wiring (heat tracing not required; 2nd unit is for back-up).
9. Install Leachate Pumping System in Tank: Install leachate transfer pump and exterior discharge piping with quick couple disconnect to load leachate into tankers for hauling (NOTE: May need to evaluate submersible pump versus centrifugal split casing transfer pump).
10. HDPE Liner Inside Tank: Clean and acid wash tank, install 40 ml HDPE liner inside tank.

TABLE 2 (Cont'd)

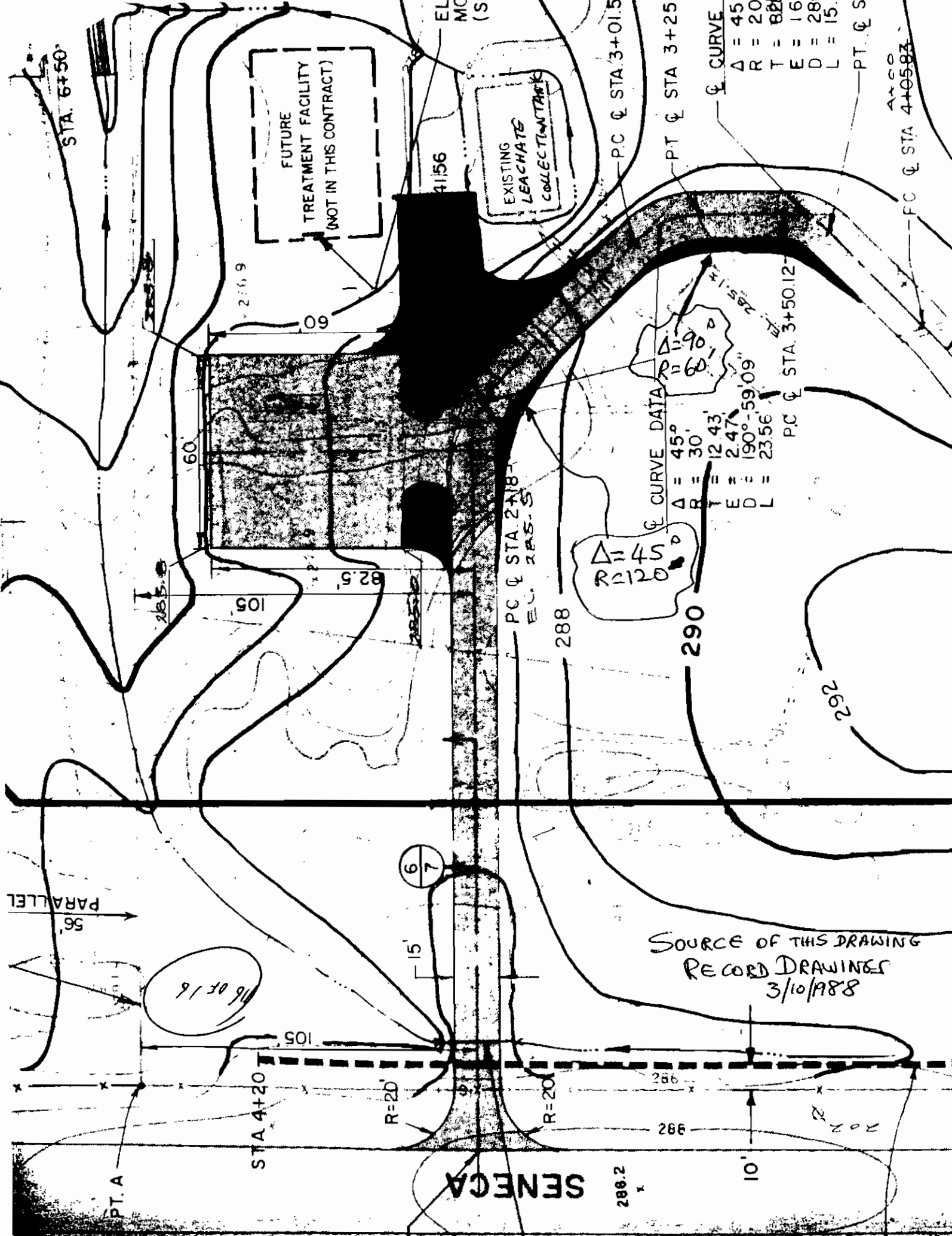
11. **Additional Heat:** Install two XP 35 MBH electric heaters with thermostats, wiring and controls (NOTE: electrical consumption may require an assessment of load availability on the incoming electrical service).

B. DECON PAD AND ACCESS

1. **Loading Platform:** Remove topsoil, build asphalt pad with drain to collection tank, and fill.
2. **Widen Road:** Widen road in areas shown on attached sketch.

C. ELECTRICAL SYSTEM

1. **Line from Main Road to Pump:** Install new service riser with line, panel board and breaker.
2. **Strip Pad Heater:** Install heater pads in all panels.
3. **Ampmeter for Well Pumps:** Install ammeters for all four leachate collection well pumps and transfer pump in tank.



STA. 6+50

FUTURE TREATMENT FACILITY (NOT IN THIS CONTRACT)

EXISTING LEACHATE COLLECTION TANK

EL MC (S)

4156

PC & STA 3+01.5

PT & STA 3+25

PC & CURVE

Δ = 45°
R = 20
T = 82.8
E = 16.1
D = 28.6
L = 15

PT. & S

ANCO

PC & STA 4+05.83

2:6.9

0.9

60

105

82.5

285.5

PC & STA 2+18
EL. 285.5

288

Δ = 45°
R = 120

PC & CURVE DATA

Δ = 45°
R = 30
T = 12.43
E = 2.47
D = 190°-59'09"
L = 23.56

PC & STA 3+50.12

290

292

PARALLEL
56'

1/6 OF 16

SOURCE OF THIS DRAWING
RECORD DRAWING
3/10/1988

STA 4+20

R=20'

R=20'

SENECA

286.2

10'

286

286

PT. A

New York State Department of Environmental Conservation
50 Wolf Road, Albany, New York 12233-7010



Thomas C. Jorling
Commissioner

MAY 16 1991

Dharmarajan R. Iyer, Ph.D
Project Manager
URS Consultants, Inc.
282 Delaware Avenue
Buffalo, NY 14202-1207

RE: Pollution Abatement Service (O&M) Site #7-38-001
Establishing Monuments and Horizontal Control

Dear Mr. Iyer:

This is to confirm our discussions of May 15, 1991 that the survey work at the above mentioned site be kept in abeyance until a decision on the construction of an on-site leachate treatment facility is made.

If you have any questions, please call me at 518/457-0927.

Sincerely,

A. K. Gupta, P.E.
Environmental Engineer 2
Operation & Maintenance Section
Bureau of Construction Services
Division of Hazardous Waste Remediation

cc: G. Rider
D. Smith
R. Lupe

a:monuhor.pas:AKG:et



AN INTERNATIONAL PROFESSIONAL SERVICES ORGANIZATION

April 24, 1991

URS CONSULTANTS, INC.

282 DELAWARE AVENUE
BUFFALO, NEW YORK 14202-1805
(716) 856-5636
FAX: (716) 856-2545

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RECEIVED
APR 24 1991

Mr. A. K. Gupta, P.E., Project Manager
Bureau of Western Remedial Action
Division of Hazardous Waste Remediation
New York State Department of
Environmental Conservation
50 Wolf Road
Albany, New York 12233-7010

PAS SITE O & M - SITE NO. 7-38-001 (W.A. D002340-8)
ESTABLISHING MONUMENTS AND HORIZONTAL CONTROL

Dear Mr. Gupta:

As per our previous discussions, we request an amendment to the approved Work Plan and Budget to include the following items as part of the initial survey of monitoring wells at the site:

- a) Re-establish horizontal baseline on the driveway.
- b) Establish two (2) references for horizontal and vertical control.
- c) Tie the two references to USGS datum.

The baseline in previous drawings from the RI/FS and site remediation drawings along East Seneca Street and was not tied into any other datum. We request to re-establish the horizontal baseline parallel to East Seneca Street along the access driveway within the fence, but outside the limit of the well cap. Two concrete monuments, each on either side of the access driveway along the horizontal baseline, will be installed as vertical controls. These two reference points will also be tied to a USGS datum sea level.

We estimate an additional budget of \$2,441.39 to accomplish the above items. A detailed breakdown of this request for a budget increase is provided in the attached table. The initial survey of the monitoring wells was included in the approved budget.

Please let me know if you have any questions or wish to discuss this request in further detail. Thank you.

Very truly yours,

URS CONSULTANTS, INC.

Arman B. Jaffer
Arman B. Jaffer, Ph.D.
Director

*USGS Datum?
State coordinate system?*

DRI/ys
Attachment

APP-24-91L.AKG

cc: Mr. John Gorton - URS
File: 35236.00 (1000)

Post-It™ brand fax transmittal memo 7671

To	AK Gupta	From	D Iyer
Co.	DEC	Co.	URS
Dept.		Phone #	
Fax #	(518) 457-3972	Fax #	

TABLE

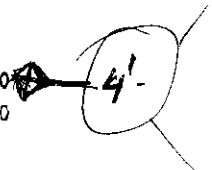
POLLUTION ABATEMENT SERVICES O & M (WA #D002340-8)
 TASK 3- ESTABLISH CONTROL AND MONUMENT POINTS

DIRECT LABOR COSTS:		YEAR 1990		YEAR 1991		YEAR 1992		YEAR 1993		DIRECT	
PROFESSIONAL LEVEL	LABOR CLASSIFICATION	No. of HOURS	DIRECT SALARY RATE	No. of HOURS	DIRECT SALARY RATE	No. of HOURS	DIRECT SALARY RATE	No. of HOURS	DIRECT SALARY RATE	TOTAL HOURS	SALARY COST
PROJECT DIRECTOR	IX		\$36.30		\$38.12		\$40.02		\$42.02		\$0.00
PROGRAM MANAGER	VIII		\$33.64		\$35.32		\$37.09		\$38.94		\$0.00
PROJECT MANAGER	VII		\$27.21	4	\$28.57		\$30.00		\$31.50	4	\$114.28
ENGINEER 4	VI		\$21.84	4	\$22.93		\$24.08		\$25.28	4	\$91.72
ENGINEER 3	V		\$18.24		\$19.15		\$20.11		\$21.12		\$0.00
DRAFTSMAN	II		\$10.57	44	\$11.10		\$11.66		\$12.24	44	\$488.40
TECHNICIAN 2	II		\$10.57		\$11.10		\$11.66		\$12.24		\$0.00
SUBTOTALS		0		52		0		0		52	\$694.40

TRAVEL AND SUBSISTANCE:

	No. of	UNITS	UNIT	TOTAL
ATR FARE	0	EACH	\$300.00	\$0.00
CAR RENTAL	0	DAYS	\$50.00	\$0.00
MILEAGE	400	MILE	\$0.23	\$92.00
PER DIEM	4	DAYS	\$66.00	\$264.00
TOLLS, ETC.	-	-	-	\$10.00
SUBTOTAL				\$366.00

EQUIPMENT

ITEM	No.	UNITS	COST	
POST HOLE DIGGER -	4	DAY	\$20.00	
HEALTH & SAFETY-LVL D+	8	MAN/DAY	\$26.00	
SUBTOTAL				\$288.00

A. TOTAL DIRECT LABOR COSTS	\$694.40
B. INDIRECT LABOR COSTS @ 134%	\$930.50
C. FEE ((A + B) * 10%)	\$162.49

TOTAL TASK LABOR \$1,787.39

TOTAL TRAVEL AND SUBSISTANCE: \$366.00

TOTAL EQUIPMENT: \$288.00

TOTAL SUBCONTRACTOR: \$0.00

TOTAL TASK COST: \$2,441.39



AN INTERNATIONAL PROFESSIONAL SERVICES ORGANIZATION

April 24, 1991

URS CONSULTANTS, INC.
282 DELAWARE AVENUE
BUFFALO, NEW YORK 14202-1805
(716) 856-5636
FAX: (716) 856-2545

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Vermont
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Wyoming

APR 30 1991

Mr. A. K. Gupta, P.E., Project Manager
Bureau of Western Remedial Action
Division of Hazardous Waste Remediation
New York State Department of
Environmental Conservation
50 Wolf Road
Albany, New York 12233-7010

RE: PAS SITE O & M - SITE NO. 7-38-001 (W.A. D002340-8)
ESTABLISHING MONUMENTS AND HORIZONTAL CONTROL

Dear Mr. Gupta:

As per our previous discussions, we request an amendment to the approved Work Plan and Budget to include the following items as part of the initial survey of the monitoring wells at the site:

- a) Re-establish horizontal baseline on the driveway.
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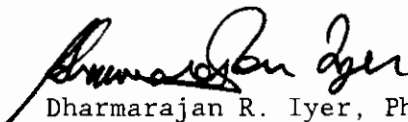
The horizontal baseline in previous drawings from the RI/FS and site remediation were located along East Seneca Street and was not tied into any other system. We propose to re-establish the horizontal baseline parallel to East Seneca Street and across the access driveway within the fence, but outside the limits of the containment cell cap. Two concrete monuments, each on either side of the access driveway along the horizontal baseline, will be installed as vertical controls. These two reference points will also be tied to a USGS datum sea level.

We estimate an additional budget of \$2,441.39 to accomplish the above three items. A detailed breakdown of this request for a budget increase is provided in the attached table. The initial survey of the monitoring wells was included in the approved budget.

Please call me if you have any questions or wish to discuss this request in further detail. Thank you.

Very truly yours,

URS CONSULTANTS, INC.


Dharmarajan R. Iyer, Ph.D.
Task Manager

DRI/ys
Attachment

4-24-91L.AKG

cc: Mr. John Gorton - URS
File: 35236.00 (1000)

TABLE

POLLUTION ABATEMENT SERVICES O & M (WA #D002340-8)
 TASK 3: ESTABLISH CONTROL AND MONUMENT POINTS

DIRECT LABOR COSTS:		YEAR 1990		YEAR 1991		YEAR 1992		YEAR 1993		DIRECT	
PROFESSIONAL LEVEL	LABOR CLASSIFICATION	No. of HOURS	DIRECT SALARY RATE	No. of HOURS	DIRECT SALARY RATE	No. of HOURS	DIRECT SALARY RATE	No. HOURS	DIRECT SALARY RATE	TOTAL HOURS	DIRECT SALARY COST
PROJECT DIRECTOR	IX		\$36.30		\$38.12		\$40.02		\$42.02		\$0.00
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ENGINEER 3	V		\$18.24		\$19.15		\$20.11		\$21.12		\$0.00
DRAFTSMAN	II		\$10.57	44	\$11.10		\$11.66		\$12.24	44	\$488.40
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SUBTOTALS		0		52		0		0		52	\$694.40
TRAVEL AND SUBSISTANCE:											
		No. of	UNITS	UNIT							TOTAL
AIR FARE		0	EACH	\$300.00							\$0.00
CAR RENTAL		0	DAYS	\$50.00							\$0.00
MILEAGE		400	MILE	\$0.23							\$92.00
PER DIEM		4	DAYS	\$66.00							\$264.00
TOLLS, ETC.		-	-	-							\$10.00
SUBTOTAL											\$366.00
EQUIPMENT											
ITEM		No.	UNITS	COST							
PDST HOLE DIGGER		4	DAY	\$20.00							\$80.00
HEALTH & SAFETY-LVL O+		8	MAN/DAY	\$26.00							\$208.00
SUBTOTAL											\$288.00
A. TOTAL DIRECT LABOR COSTS											\$694.40
B. INDIRECT LABOR COSTS @ 134%											\$930.50
C. FEE ((A + B) * 10%)											\$162.49
TOTAL TASK LABOR											\$1,787.39
TOTAL TRAVEL AND SUBSISTANCE:											\$366.00
TOTAL EQUIPMENT:											\$288.00
TOTAL SUBCONTRACTOR:											\$0.00
TOTAL TASK COST:											\$2,441.39

URS

AN INTERNATIONAL PROFESSIONAL SERVICE ORGANIZATION

URS CONSULTANTS, INC.

570 DELAWARE AVENUE
BUFFALO NEW YORK 14202-1207
(716) 883-5525
FAX (716) 883-0754

January 16, 1991

Mr. A. K. Gupta, P.E., Project Manager
Bureau of Western Remedial Action
Division of Hazardous Waste Remediation
NYS Department of Environmental Conservation
50 Wolf Road
Albany, New York 12233-7010

RE: POLLUTION ABATEMENT SERVICES O&M; W.A. D002340-8
INTERIM PLAN AND FACILITY EVALUATIONS

ATLANTA
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DENVER
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SAN FRANCISCO
SAN RAMON
SAN JOSE
WASHINGTON

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JAN 18 1991

OPERATIONS & MAINTENANCE
SECTION

Dear Mr. Gupta:

Thank you for your letter of December 28, 1990 with comments on our Interim Plan for leachate level control and on our evaluations of the leachate collection system and the containment cell cap, all submitted under Tasks 2 and 3 of the subject work assignment. We have reviewed your letter and a meeting has been scheduled to discuss specific details related to key issues. Several of those are described in the following paragraphs.

Interim Plan

URS was under the impression (possibly mistaken), based upon your letter of November 13, that the leachate level inside the cell was critically high and should be immediately reduced to a point where the pressure gradients would be such as to induce flow inward. Our plan was developed with that as a primary objective. The secondary objective of the plan is to obtain some indication, other than theoretical, of leachate volume, sustainable pumping rates, and inflow to the cell. That information is required to replace the theoretical aspects of the present assessment of the economics of on-site treatment versus off-site disposal. Although removal rates may be lowered to any level desired, we should agree upon a program which will provide the data necessary for an assessment.

Leachate Holding Tank

A program to drain and clean the leachate holding tank, so that a thorough inspection can be accomplished, is currently in progress. Until that inspection is complete, URS does not wish to speculate on details of repair and/or modification.

On-Site Treatment

As we understand the scope of the work assignment, one of the tasks is to come to a final determination of the cost-effectiveness of on-site treatment as opposed to off-site disposal. The conceptual evaluation presented by URS is based on theoretical procedures. The inference drawn from comments in the letter is that on-site treatment has been selected. If that is so, we can omit the evaluation step and move directly to unit design.

Mr. A. K. Gupta
January 16, 1991
Page 2

Leachate Control

There appear to be differences in conclusions drawn from the interpretation of available data relating to leachate at the site. URS does not believe that the sporadic pumping of small quantities of leachate, since remediation was completed, has produced data adequate for characterization of site leachate volume or inflow. At this point, the use of conclusions drawn in conceptual work contained in the RI/FS report is probably not appropriate in as much as the selected remediation differs from the conceptual design. During our meeting, we need to arrive at a mutually acceptable program for leachate control during the O&M portion of this work assignment.

Very truly yours,

URS CONSULTANTS, INC.

for Charles W. Hurley
Dharmarajan R. Iyer, Ph. D.
Task Manager

DRI/ys

1-16-91.AKG

cc: P. David Smith - NYSDEC
Gerald Rider - NYSDEC
Raymond Lupe - NYSDEC
John Gorton - URS
Charles Hurley - URS
File: 35236.00 (1000)

TELECON REPORT

FROM : A.K. Gupta of ~~the~~ O&M Section

TO : Dick Brazell of Region - 7

DATE : 12/19/90 SUB PAS site.

BACKGROUND/SUMMARY : The details on Record Drawings indicate that ~~the~~ a sand cushion has been provided under HDPE liner & top of slurry wall providing a preferential path for escape of leachate. Also this sand cushion may result in very high infiltration through cap.

TELECON : I Requested Dick to explain me about the details of connection (as build) between slurry wall & cap. Also a request was made to provide quality of sand used as cushion.

Dick responded that he will Fax & Mail the Spec. of sand used on site. Also he mentioned that ~~the~~ sand cushion extends over the slurry wall. ~~The~~ Dick also mentioned that in the past ~~the~~ when construction was completed & leachate pumping was never done due to fueling shortage the leachate ~~is~~ levels within cell were high enough to force leachate above the slurry wall & below the liner. He mentioned that in past leachate has leaked ~~from~~ out ~~from~~ between slurry wall & cap. but now the leachate levels are lower than these old levels. It is possible that leakage is still occurring at lower section of slurry wall.

Copy : GJR JJW FILE

POLLUTION ABATEMENT SERVICES
 (Preliminary) OPERATIONAL COST ESTIMATES

SYSTEM	FLOW RATE	CAPITAL COST	CONCRETE	AMMORTIZED ANNUAL COST
GRANULAR ACTIVATED CARBON	5 GPM	24,000	150,000	83,000
AIR STRIPPER & GRANULAR ACTIVATED CARBON	50 GPM	240,000	1,000,000	1,200,000
SEQUENCED BATCH REACTOR	50 GPM	500,000	1,500,000	1,500,000
SEQUENCED BATCH REACTOR - RENTAL	15 GPM	180,000	1,000,000	1,000,000
SEQUENCED BATCH REACTOR - SKID MOUNTED	5 GPM	20,000	150,000	116,000
OFF-SITE DISPOSAL (\$0.81/gal)	2,000 GPD		520,000	520,000
OFF-SITE DISPOSAL (\$0.81/gal)	20,000 GPD		5,200,000	5,200,000

NOTES: COST IS AMMORTIZED OVER TEN (10) YEARS AT 10%

Page 1

POLLUTION ABATEMENT SERVICES
(Preliminary) OPTION COST ESTIMATES

SYSTEM	FLOW RATE	CAPITAL COST	O&M COST	AMMORTIZED ANNUAL COST
GRANULAR ACTIVATED CARBON	5 GPM	200,000 - 700,000	50,000 - 150,000	83,000 - 264,000
AIR STRIPPER & GRANULAR ACTIVATED CARBON	50 GPM	500,000 - 1,200,000	100,000 - 200,000	181,000 - 395,000
SEQUENCED BATCH REACTOR	50 GPM	500,000 - 800,000	75,000 - 150,000	156,000 - 280,000
SEQUENCED BATCH REACTOR - RENTAL	15 GPM	-	180,000 - 400,000	180,000 - 400,000
SEQUENCED BATCH REACTOR - SKID-MOUNTED	15 GPM	250,000 - 600,000	75,000 - 150,000	116,000 - 248,000
OFF-SITE DISPOSAL (\$0.81/gal)	2,000 GPD	-	520,000	520,000
OFF-SITE DISPOSAL (\$0.81/gal)	20,000 GPD	-	5,200,000	5,200,000

NOTES: COST IS AMMORTIZED OVER TEN (10) YEARS AT 10%

URS

What is area of Cap - P.S. gains

A.K. GUPTA

New York State Department of Environmental Conservation
50 Wolf Road, Albany, New York 12233



Thomas C. Jorling
Commissioner

DEC 28 1990

Dharmarajan R. Iyer, Ph. D.
Task Manager
URS Consultants, Inc.
570 Delaware Avenue
Buffalo, NY 14202

RE: Pollution Abatement Services O&M
Interim Plan to Lower Leachate Levels, and
Evaluation of Leachate Collection System and Containment Cell Cap

Dear Mr. Iyer: *Pharma*

The URS's submittal for Interim Plan to lower the leachate level within the Pollution Abatement Services (PAS) containment cell was received on November 26, 1990. Thereafter the draft report on Task 2 and 3, Evaluation of Leachate Collection System and Containment Cell Cap was received on December 3, 1990. The Department has completed its review of the above mentioned submittals and has the following comments:

Interim Plan:

The Department agrees that the removal of leachate from the containment cell should begin as soon as possible but at a realistic rate. We strongly feel that the volume of leachate to be removed in the time frame proposed in your "Interim Plan" is neither cost effective nor feasible. The leachate removal should be done while an on-site treatment system is being implemented. During April-May 1990 a total of about 56,000 gallons of leachate was pumped out from the site and the monthly groundwater elevations within the containment cell showed a drop of 0.91' to 3.50' with a time lag varying from one to six months depending upon monitoring well location. Also it was observed that the groundwater elevations within the cell between 12/89 to 12/90 remained basically unchanged in spite of removal of 56,000 gallons of leachate. This indicates that in one year, at the existing groundwater elevations, about 56,000 gallons of inflow occurred. This assumes that no other water was leached out the site.

Keeping the above in mind, the Department suggests that consideration be given to removing 50,000 gallons of leachate in the first month and thereafter 10,000 gallons every month until on-site system is made operative. This will conform to the conditions of the past year and we believe maintain the water level below the lowest top elevation of the slurry wall (near well SWW 11). Also it is recommended that the leachate removal should be scheduled during the second week of each month, the department will take the groundwater elevations during the first week of

each month. The Department does not agree to the need to take groundwater elevations at an increased frequency. The URS's submittal of quotes for leachate disposal was received on December 21, 1990 and is being reviewed by the Department.

Comments on Task 2 and 3 Report:

1. Page 1. The site visit was done on October 24, 1990. Please correct the date at all locations in the report.
2. Page 2. On-site inspection of the leachate holding tank has determined that it is in poor condition and probably leaks. The concrete tank should be sealed or perhaps a liner can be installed to eliminate potential leakage. The tank repair can be undertaken when the tank is cleaned. Also the tank roof and sides should be insulated at the same time to at-least four (4) feet below ground or up to the top of the HDPE liner on the outside of the tank. The tank repair will allow the use of more of the tanks volume (that portion above the HDPE liner elevation).
3. Page 3. The automatic control system in the leachate recovery wells should be checked and repaired through the general operation and maintenance activities at this site. These types of activities may be included in a general electrical/mechanical portion of O&M subcontract for this site.
4. Page 4. Once the leachate holding tank is repaired and insulated the freezing problem will be less severe. The URS should give consideration to providing heat with thermostat control thus eliminating freeze and thaw damage to the tank and assures proper operation of the equipment.
5. Page 6. According to the historical records the leachate was pumped as follows:

291580 gal.

<u>Period</u>	<u>Quantity (Gallons)</u>	<u>Contractor</u>
1988	391,000	Sevenson
8/88 to 11/88	14,912	Frontier Chemical
8/89	15,000(approx)	Frontier Chemical
4/90	4,000(approx)	Frontier Chemical
5/90 to 6/90	52,144	Environmental Prod.

It appears that the total volume of leachate to be pumped out from the site to lower the leachate elevations to 264' may be close to six (6) million gallons. The time frame as suggested by URS does not seem to be realistic.

The recharge rate of the leachate trench appears to be approximately 1,000 gallons per day. URS in previous reports has used a recharge rate similar to this. In the RI/FS report (page 103) URS estimated that the recharge rate would allow a removal rate of 250,000 gallons/yr (960 gal/weekday) and stated that the average rate of leachate generation would decline with time as the water table

elevation decreased. On page 116 of the RI/FS, the treatment system is designed for 24,000 gallons every 5 day week (960 gal/weekday).

This rate of recharge is also suggested by field measurements. On June 30, 1987 leachate was pumped for 4 hours. Approximately 36,000 gallons of leachate was removed from the collection trench. Leachate levels were still 0.8 ft. lower when measured 14 days later. Levels did not fully recover until August 4th, 36 days after pumping.

From this information it appears that the existing collection system cannot sustain the pumping rates in the new URS proposal and that the recharge rates in the earlier URS reports seem more accurate.

6. Page 7. See comment # 5 above. The cap as designed and constructed, is not the the current state-of-the-art type (i.e. HDPE liner and clay layer underneath). The sand layer installed under the HDPE liner at the PAS site will not contribute to the cap's ability to prevent precipitation and snow melt from entering the landfill. With this in mind, the URS should re-evaluate their assumption that infiltration through the cap is insignificant. Perhaps, in long term, reconstruction of the cap should be considered with the intent to reduce the leachate production.
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8. Page 9. The sizing of an on-site treatment unit should be done on the basis of the most economical capitalized cost for the life of the project.
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11. Page 15. The site visit was done on October 24, 1990.
12. Page 16. Item 7; Complete the sentence "due to previous days rains."
13. Page 17. Item 16; What are sun boots?

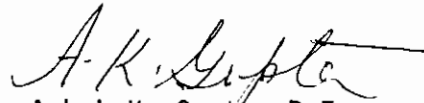
14. Appendix D. Page I-2; Date of observation should be included in the table. Also the summary should be updated to include latest data. Enclosed is a copy of groundwater elevation data from 4/89 thru 12/90 (see attachment 1).

Groundwater Volumes and Flow Calculations. See comment # 6 above. The calculations to determine flow thru the slurry wall assumed a thickness of three (3) feet while the wall is actually at least four (4) feet thick (Final Engineering Report, Page 28). The calculations to determine flow thru the lodgement till did not take into account that this till exhibits vertical permeability anistorophy and flow rates may in fact be much lower than calculated. The URS's estimates as submitted now are significantly different then their past estimates as per RI/FS August 1985, and Evaluation of Treatment Alternatives October 1985. Which estimate should be adopted and why?

16. Appendix F. See comment # 7 above.
17. Appendix I. The area between the pavement in front of the storage shed and the access road should be squared off for ease of snow plowing (see attached drawing).

If you have any questions, please call me at (518)457-0927.

Sincerely,



Ashok K. Gupta, P.E.
Environmental Engineer II
Operation and Maintenance Section
Div. of Hazardous Waste Remediation

Attachments

cc: R. Lupe
G. Rider

PAS GROUNDWATER ELEVATIONS LONG-TERM MONITORING WELLS

date	SW1		SW2		SW3		SW4		SW5		SW6		SW7		SW8		SW9		SW10		SW11		SW12	
	for	elev	for	elev	for	elev	for	elev	for	elev	for	elev	for	elev	for	elev	for	elev	for	elev	for	elev	for	elev
04/06/89	-8.94	280.39	-14.54	274.83	-15.15	271.35	-12.81	270.19	-8.36	268.65	-6.91	265.15	-5.18	272.75	-3.94	274.30	-15.63	270.00	-9.79	270.64	-5.33	268.17	-7.21	265.61
05/04/89	-9.50	279.82	-14.42	274.95	-15.00	271.49	-14.79	268.81	-8.67	268.35	-7.25	265.81	-5.39	272.54	-4.25	273.99	-15.71	268.84	-11.18	269.25	-5.37	268.13	-7.56	265.26
06/05/89	-9.50	279.82	-14.16	275.21	-14.67	271.93	-15.08	268.52	-7.79	269.23	-7.21	265.85	-5.18	272.75	-4.06	274.18	-15.43	270.12	-11.39	269.04	-4.91	268.59	-7.69	265.13
09/11/89	-10.44	278.89	-14.71	274.66	-14.75	271.75	-15.92	267.68	-9.33	267.69	-7.75	265.31	-6.22	271.71	-6.31	271.93	-15.92	269.63	-14.66	269.77	-5.71	267.79	-7.94	264.88
10/25/89	-10.13	279.20	-14.92	274.45	-14.92	271.58	-14.60	269.00	-8.67	268.35	-6.33	266.73	-6.26	271.67	-4.56	273.68	-16.38	269.17	-13.85	266.58	-5.39	268.11	-6.27	265.55
11/12/89	-9.75	275.58	-14.58	274.79	-14.67	271.83	-13.54	270.05	-10.67	266.35	-7.75	265.31	-5.55	272.38	-4.31	273.93	-15.92	269.63	-10.60	269.83	-5.54	266.96	-7.69	265.13
12/28/89	-9.92	279.41	-14.38	274.49	-15.08	271.42	-15.56	268.04	-8.83	268.19	-8.21	264.85	-5.64	272.29	-4.44	273.80	-15.84	269.71	-12.43	268.00	-5.42	268.08	-7.85	264.97
01/7/90	-8.75	280.58	-14.75	274.62	-15.00	271.50	-13.42	270.18	-8.45	268.56	-7.44	265.62	-5.27	272.66	-3.67	274.57	-15.50	270.05	-10.00	270.43	-5.16	268.34	-7.35	265.47
02/14/90	-8.29	281.04	-14.46	274.91	-14.92	271.58	-13.54	270.06	-8.67	268.35	-7.71	265.35	-5.46	272.47	-3.92	274.32	-15.25	270.30	-10.15	270.27	-5.21	268.29	-8.00	264.82
03/28/90	-8.67	280.56	-14.14	275.23	-14.67	271.83	-14.25	269.35	-8.33	268.69	-7.75	265.31	-5.08	272.85	-4.16	274.08	-14.92	270.63	-10.59	269.74	-4.79	268.71	-8.12	264.70
04/24/90	-8.54	280.79	-13.92	275.45	-14.38	272.12	-13.71	269.35	-7.67	268.69	-6.75	265.64	-4.92	273.01	-3.79	274.45	-14.75	270.80	-10.44	269.99	-4.25	269.25	-7.42	265.40
05/07/90	-9.16	280.17	-13.83	275.54	-14.53	271.87	-14.63	268.97	-8.00	269.02	-7.42	265.64	-4.92	273.01	-3.67	274.57	-15.00	270.55	-11.42	269.01	-4.83	268.67	-7.42	265.40
06/11/90	-9.42	279.91	-14.00	275.37	-14.69	271.81	-15.38	268.22	-9.50	267.52	-7.75	265.31	-5.38	272.55	-4.42	273.82	-15.16	270.39	-12.46	267.97	-5.42	268.08	-7.54	265.28
07/18/90	-10.31	279.02	-14.33	275.04	-14.67	271.83	-15.63	267.97	-8.38	268.64	-7.67	265.39	-5.88	272.05	-5.33	272.91	-15.12	270.43	-13.88	266.55	-4.92	268.58	-7.69	265.13
08/21/90	-10.67	278.66	-14.88	274.49	-14.84	271.65	-15.08	268.52	-8.42	268.60	-7.58	265.48	-8.42	269.51	-6.83	271.41	-16.00	269.55	-15.00	265.43	-4.90	268.50	-7.61	265.21
09/27/90	-11.00	278.33	-15.33	274.04	-15.04	271.65	-15.96	267.64	-8.69	268.33	-8.00	265.06	-7.08	270.85	-7.92	270.32	-16.50	269.05	-15.88	264.55	-5.18	268.32	-8.21	264.61
10/15/90	-10.04	279.23	-15.29	274.08	-15.08	271.21	-14.46	269.14	-8.00	269.02	-6.71	266.35	-6.48	271.45	-4.52	273.72	-16.60	268.95	-14.67	266.76	-5.12	268.38	-6.94	265.88
11/26/90	-9.35	279.98	-14.92	274.45	-15.16	271.34	-13.75	269.85	-9.08	267.94	-7.29	265.77	-6.21	271.72	-4.23	274.01	-15.08	269.47	-11.08	269.35	-5.67	267.83	-7.60	265.22
12/10/90	-8.77	280.56	-14.83	274.54	-15.00	271.50	-13.10	270.50	-8.67	268.35	-6.75	265.31	-5.75	272.18	-3.92	274.32	-15.77	269.78	-10.31	270.12	-5.16	268.34	-7.21	265.61

ATPCHEMNET-1

PAS GROUP

date	LS2 ft. 287.5 elev 289.81	LS2 ft. 287.1 elev 289.72	LS2 ft. 287.5 elev 289.85	LS3 ft. 275.8 elev 278.62	LS3 ft. 275.5 elev 278.06	LS4 ft. 276.3 elev 279.25	LS5 ft. 270.2 elev 272.94	LS6 ft. 271.4 elev 274.14	LS6 ft. 270.9 elev 274.03	LS6 ft. 270.9 elev 274.39	LS8 ft. 299.9 elev 272.83	LS8 ft. 290 elev 273.42
04/06/89	-5.75	-8.57	-14.89	-4.38	-8.74	-9.41	-9.21	-8.19	-8.42	-10.64	-5.52	-7.64
05/04/89	284.06	281.16	274.96	274.24	268.86	268.48	265.42	265.51	264.28	263.13	266.31	264.70
06/05/89	-6.50	-8.77	-15.15	-4.67	-9.32	-10.77	-1.69	-8.79	-8.58	-11.30	-6.89	-8.80
09/11/89	-9.54	-10.07	-16.76	-6.71	-10.90	-13.18	-7.96	-9.27	-8.38	-12.39	-7.14	-10.26
10/25/89	-6.17	-5.80	-15.68	-4.96	-9.51	-12.75	-5.15	-7.54	-7.58	-10.84	-5.66	-8.39
11/12/89	-5.25	-8.32	-14.60	-4.63	-9.32	-10.23	-7.77	-9.21	-8.75	-11.51	-5.48	-8.64
12/28/89	-7.67	-8.61	-14.93	-4.79	-9.82	-11.35	-7.36	-9.54	-9.25	-11.80	-7.23	-9.51
01/17/90	-4.16	-6.25	-13.92	-4.00	-9.00	-9.38	-7.21	-8.27	-8.88	-11.00	-6.00	-7.92
02/14/90	-4.79	-6.75	-13.83	-4.33	-8.92	-9.54	-7.92	-9.16	-9.00	-11.16	-5.58	-8.58
03/28/90	-6.46	-7.42	-14.00	-4.46	-9.00	-9.71	-8.25	-8.65	-9.54	-10.83	-6.46	-9.54
04/24/90	-5.88	-7.16	-13.83	-4.21	-8.83	-9.11	-7.58	-8.65	-8.58	-10.83	-5.58	-8.18
05/07/90	-4.90	-7.12	-14.16	-3.92	-8.88	-10.58	-7.67	-9.16	-8.58	-11.00	-5.58	-8.18
06/11/90	-7.42	-7.92	-14.08	-4.67	-9.25	-11.42	-7.67	-9.40	-9.60	-11.25	-5.77	-8.71
07/18/90	-9.25	-8.55	-15.58	-7.08	-10.58	-12.88	-7.83	-9.23	-8.31	-12.50	-7.16	-10.67
08/21/90	-9.16	-8.75	-18.04	-5.71	-11.12	-13.46	-7.83	-9.40	-8.31	-12.75	-7.54	-10.83
09/27/90	-8.58	-8.63	-16.23	-8.29	-11.75	-14.21	-8.33	-10.00	-9.04	-13.18	-7.23	-11.06
10/15/90	-4.85	-7.27	-15.06	-5.02	-10.00	-13.38	-7.12	-8.19	-8.21	-11.83	-4.88	-9.58
11/26/90	-5.25	-7.27	-14.21	-4.58	-9.60	-10.48	-7.67	-8.96	-8.58	-12.04	-5.29	-9.50
12/10/90	-5.08	-7.08	-12.58	-4.08	-9.23	-9.77	-7.44	-8.63	-8.42	-11.54	-5.12	-8.79

PAS GROUP

	date	LS9	81.274
		top	elw
			216.72
4	04/06/89	-8.27	268.45
5	05/04/89	-8.34	268.36
6	06/05/89	-8.63	268.09
7			
8			
9	09/11/89	-9.53	267.09
0	10/25/89	-8.46	268.26
1	11/12/89	-8.54	268.18
2	12/28/89	-8.71	268.01
3	01/17/90	-8.21	268.51
4	02/14/90	-8.36	268.56
5	03/28/90	-8.50	268.22
6	04/24/90	-8.25	268.47
7	05/07/90	-8.42	268.30
8	06/11/90	-8.67	268.05
9	07/18/90	-8.88	267.84
0	08/21/90	-9.00	267.72
1	09/27/90	-8.79	267.93
2	10/15/90	-8.00	268.72
3	11/26/90	-7.73	268.99
4	12/10/90	-7.75	268.97

PROPOSED SECONDARY CONTAINMENT & DECON AREA (50' x 20' x 2')

EXIST LEACHATE COLLECTION TAP

P.C. & STA. 3-01.56

P.T. & STA. 3+25

Δ CURVE DATA

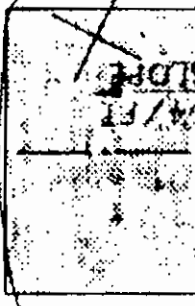
Δ = 45°
R = 20'

SWW #

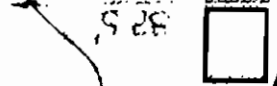
97-3+6.83

3-34.66

Space to help plow



SLOTTED W/FT



STORAGE SHED

Δ = 180°
R = 7.5'

P.C. & STA. 2+18

Δ CURVE DATA

Δ = 30°
R = 12.43'
L = 23.50'

PT = PC

Δ = 45°
R = 120'

PC & STA. 1+80

HOSE BIBB

15" DIA. IMC CULVERT

SWW 2

SWW 1

287.8

287.9

2933

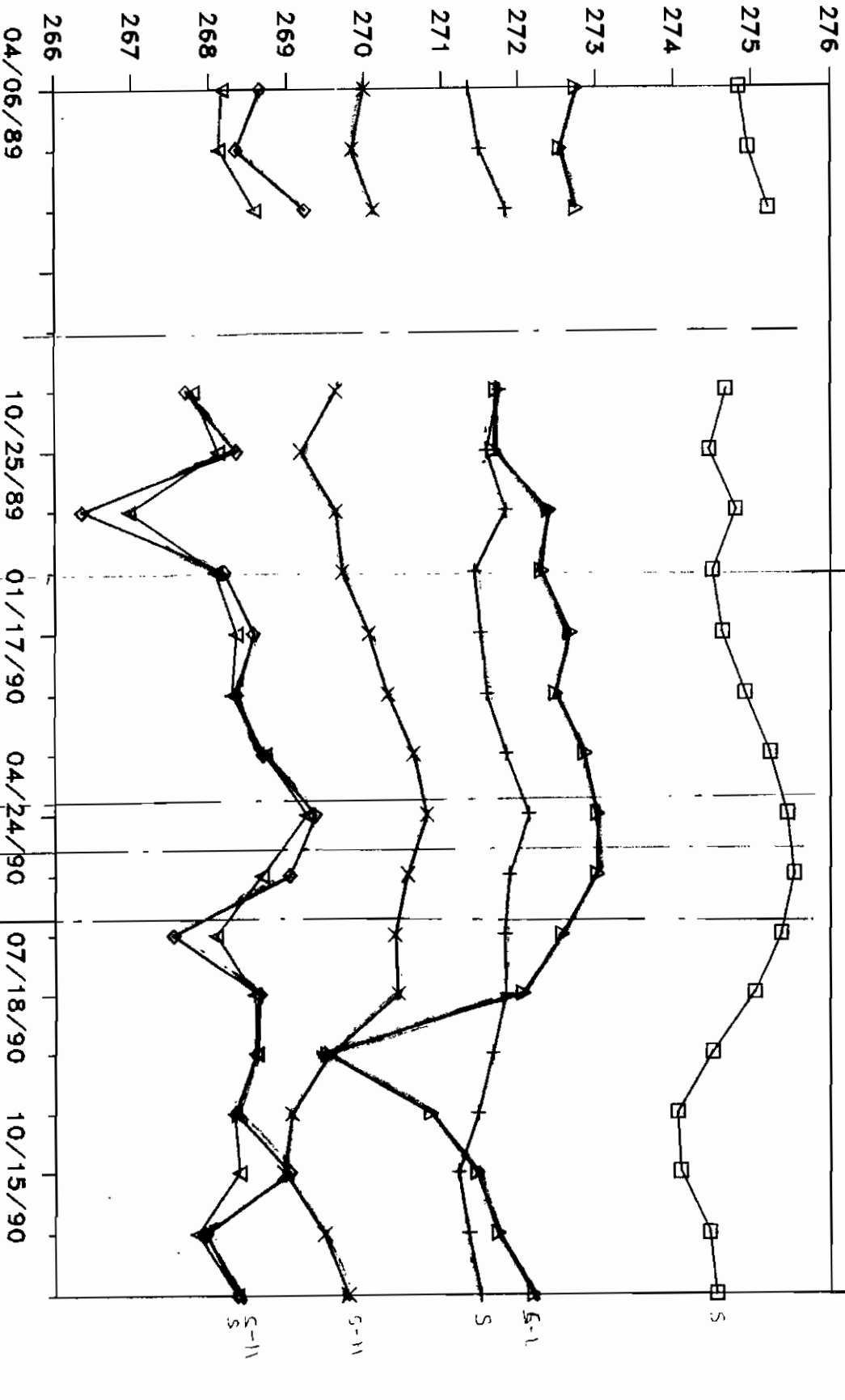
2933

GATE

PAS GW ELEVATIONS

1 YEAR (1990)

GW ELEV. (FT)



14,912 G.

15,000 G.

4000 G.

10,889 G.

41,235 G.

86,952 gal.

SWW2

SWW3

SWW5

DATE

SWW11

S-11

S-11

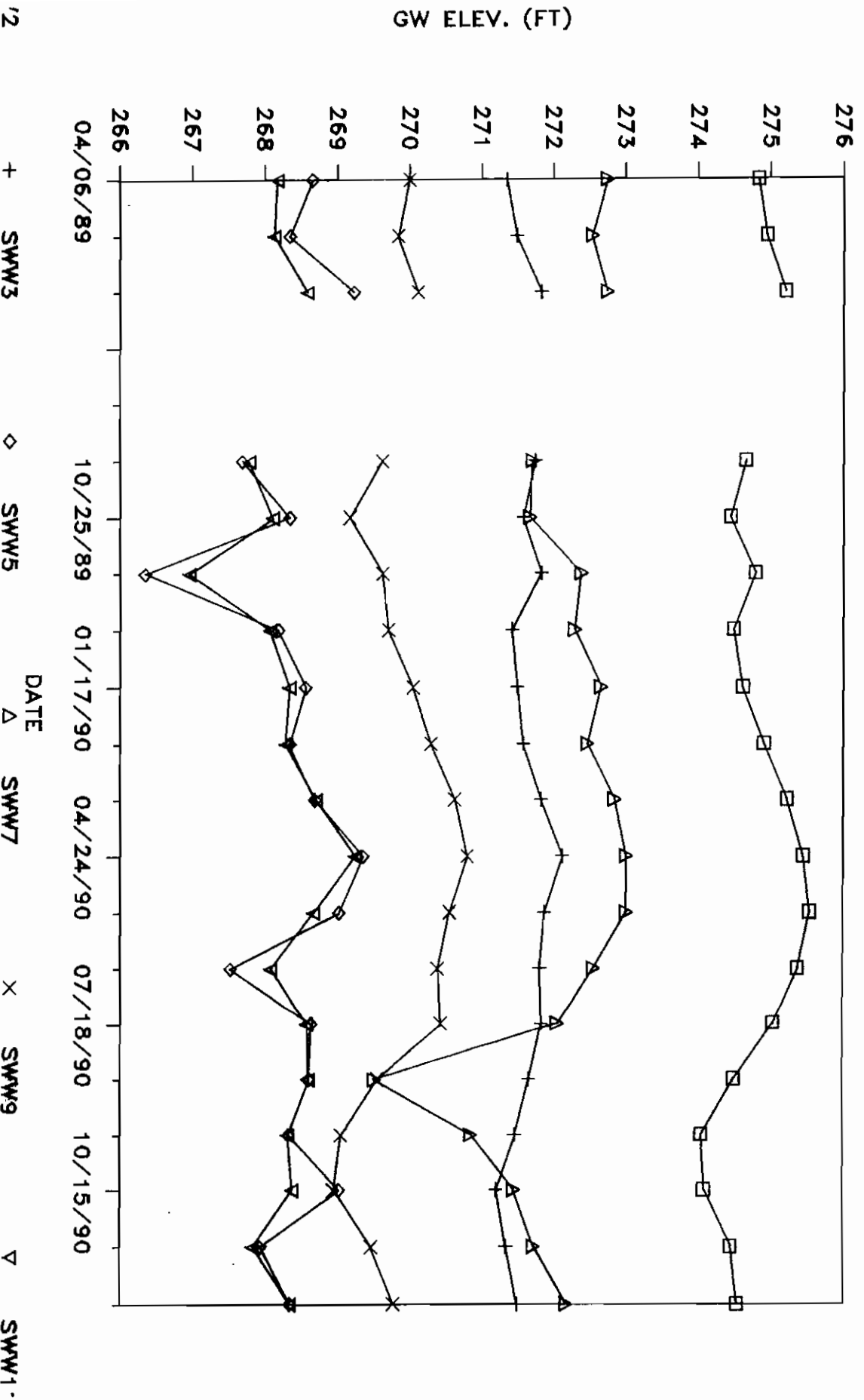
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S-1

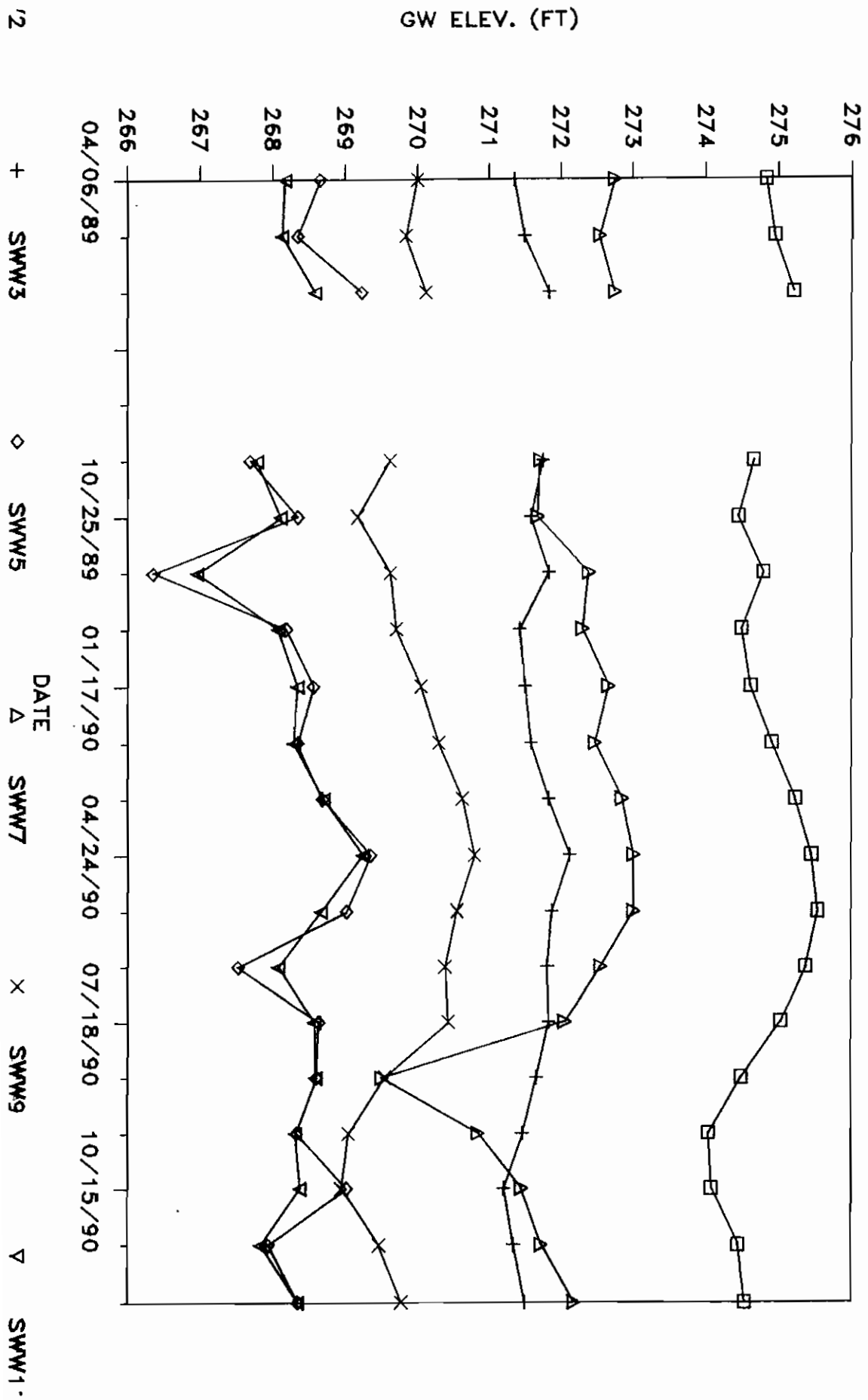
S

See page 5 for details

PAS GW ELEVATIONS



PAS GW ELEVATIONS





New York State Department of Environmental Conservation

MEMORANDUM

TO: A.K. Gupta, O&M Section
FROM: Bob Edwards, CRP Section *Bob Edwards*
SUBJECT: URS Interim Plan for the PAS Site

DATE: DEC 26 1990

I have reviewed the "Interim Plan to Lower Leachate Levels" submitted by URS.

This plan calls for the removal of 20,000 gal/weekday of leachate from the site. This is 400,000 gallons of leachate to be removed during the first 4 weeks. After this URS will modify the pumping rates if needed. A long-term rate of 2,000 to 7,000 gallons/day is recommended in this plan as the required rate needed to maintain an inward gradient across the contaminant cell.

This plan "as is" is not feasible for many reasons. The foremost reason is that the recharge rate of the leachate collection trench is nowhere near the proposed pumping rates. Any system designed to handle the pumping rates proposed would be vastly oversized.

The recharge rate of the leachate collection trench appears to be approximately 1,000 gallons per day. URS in previous reports has used a recharge rate similar to this. In the RI/FS report (page 103) URS estimated that the recharge rate would allow a removal rate of 250,000 gallons/yr (960 gal/weekday) and stated that the average rate of leachate generation would decline with time as the water table elevation decreased. On page 116 of the RI/FS, the treatment system is designed for 24,000 gallons every 5 weeks (960 gal/weekday).

This rate of recharge is also suggested by field measurements. On June 30, 1987 leachate was pumped for 4 hours. Approximately 36,000 gallons of leachate was removed from the collection trench. Leachate levels were still 0.8 ft. lower when measured 14 days later. Levels did not fully recover until August 4th, 36 days after pumping.

It is apparent that the existing collection system cannot sustain the pumping rates in the new URS proposal and that the recharge rates in earlier URS work seem more accurate. Once a regular schedule of leachate removal is started my section will be able to monitor the collection system to determine recharge rates directly.

I also have comments on the URS Evaluation of the collection system and cap. They are as follows:

Page 4 - I would recommend that a new tank be installed instead of trying to improve the existing concrete "tank". I do not believe that the existing tank was meant to be used permanently.

Page 6 - Although water levels inside the cell are higher they

have not increased significantly within the last year.

Appendix D - The calculations to determine flow thru slurry wall assumed a thickness of 3 feet while the wall is actually at least 4 feet thick (Final Engineering Report, Page 28).

The calculations used to determine flow thru the lodgement till did not take into account that this till exhibits vertical permeability anistorophy and flow rates may in fact be much lower than calculated.

Finally the size of the on-site treatment system discussed in this proposal is based on a flow rate which cannot be sustained. I recommend that URS be directed to review its past work including the 1985 Evaluation of Alternatives for Treatment of PAS Groundwater/Leachate Final Report, the RI/FS, and the Final Engineering Report of March 1988.

Please keep me informed of the status of this project and see me if you have any questions.

Comments on Task 2 and 3 Report:

1. Page 1. The site visit was done on October 24, 1990. Please correct the date at all places.
2. Page 2. On-site inspection of the leachate holding tank has determined that it is in poor shape and probably leaks. The concrete tank should be sealed or perhaps a liner can be installed to eliminate potential leakage. The tank repair can be undertaken when cleaned. Also the tank roof and sides should be insulated at the same time with at-least four (4) feet below ground or up to top of HDPE liner on the outside. The tank repair will allow the use of more of the tank (that portion above the HDPE liner elevation).
3. Page 3. The automatic control system of leachate recovery wells should be checked and repaired through the general operation and maintenance activities at this site. This type of activities may be included in a general electrical/mechanical subcontract for this site.
4. Page 4. Once the leachate holding tank is repaired and insulated the freezing problem will be less severe. The URS should consider installing a oil type radiant space heater with thermostat set at 40-45 degrees, this will keep the entire tank above freezing thus eliminating freeze and thaw damage.
5. Page 6. According to the historical records the leachate was pumped as follows:

<u>Period</u>	<u>Quantity (Gallons)</u>	<u>Contractor</u>
1988	391,000	Sevenson
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to be checked.



It appears that the total volume of leachate to be pumped out from site to lower the leachate elevations to 264' may be close to 6 million gallons. But the time frame as suggested by the URS does not seem to be realistic. See number 15 below.

6. Page 7. The long-term pumping rates as estimated in this proposal are significantly different than the similar estimates done in past studies. The cap as designed and constructed, is not the the current state-of-the-art type (i.e. HDPE liner and clay layer underneath). Instead, the sand layer installed under the HDPE liner at the PAS site will not contribute to the cap's ability to prevent precipitation and snow melt from entering the landfill. With this in mind, the URS should re-evaluate their assumption that infiltration through the cap is insignificant. Perhaps, in long term, reconstruction of the cap should be considered with the intent to reduce the leachate production.

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8. Page 9. The sizing of an on-site treatment unit should be done on the basis of the most economical capitalized cost for the life of the project.
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10. Page 12. After one time bush-hog the site should be mowed at a regular interval. Also the foliage should be trimmed back up to five (5) feet from fence. If possible, the fertilizer application should be limited to twice a year.
11. Page 15. The site visit was done on October 24, 1990.
12. Page 16. Item 7; Complete the sentence "due to previous days rains."
13. Page 17. Item 16; What are sun boots?
14. Appendix D. Page I-2; Date of observation should be included in the table. Also the summary should be updated to include latest data. Enclosed is a copy of groundwater elevation data from 4/89 thru 12/90 (see attachment 1).
15. Groundwater Volumes and Flow Calculations. The URS's estimates as submitted now are significantly different then their past estimates as per RI/FS August 1985, and Evaluation of Treatment Alternatives October 1985 (for additional comments, see attachment 2). Which estimate should be adopted and why?
16. Appendix F. See comment # 7.
17. Appendix I. The area between the pavement in front of storage shed and the access road be square off for ease of snow plowing (see attached drawing).

If you have any questions, please call me at (518)457-0927.

Sincerely,

Ashok K. Gupta, P.E.
 Environmental Engineer II
 Operation and Maintenance Section
 Div. of Hazardous Waste Remediation

attachments

cc: R. Lupe
 G. Rider

Page 7 of 3



New York State Department of Environmental Conservation

MEMORANDUM

TO: A.K. Gupta, O&M Section, BCS
FROM: Philip G. Waite, O&M Section, BCS
SUBJECT: P.A.S. Site No. 7-38-001
DATE: December 13, 1990

I have reviewed the report from URS, dated November 1990, concerning the evaluations of the leachate collection system and containment cell cap. I also reviewed the interim plan from URS, dated November 26, 1990, concerning lowering the leachate level. The following are my thoughts and comments as they relate to these reports and as they relate to the leachate quantity estimates and leachate treatment options (offsite vs. onsite):

1. On-site inspection of the leachate holding tank has determined that it is in poor shape and probably leaks. The concrete tank should first be sealed or perhaps a liner can be installed to eliminate potential leakage. This would also allow the use of more of the tank (that portion above the HDPE cap liner elevation).

In a longer term pumping scenario, I recommend construction of a secondary containment pad and the installation of additional holding tanks with a combined capacity of at least 50,000 gallons. The larger capacity will reduce the frequency of offsite transport and thus costs.

2. With the immediate need to lower the leachate levels, it would seem to me that all four pumping stations should be utilized to lower the level as fast as possible in the interim plan. URS is recommending using only three for the first four week period.
3. With your observation that the cap design may allow high leachate elevations under the HDPE liner to flow over the slurry wall through the sand layer, the down gradient slope face between the slurry wall and White Creek should be closely inspected for evidence of any seeps.
4. I agree with your comments that the cap, as designed and constructed, is not the current state-of-the-art required by EPA (ie. HDPE liner and clay layer underneath). Instead, the sand layer installed under the HDPE liner at the P.A.S. site will not contribute to the cap's ability to prevent precipitation and snow melt from entering the landfill. With this in mind, I agree with you that URS should reevaluate their assumption that infiltration through the cap is insignificant. Perhaps, in the long term, reconstruction of the cap should be considered with the intent to reduce the leachate production.
5. I believe the long term option for leachate handling at this site should be an on-site treatment and discharge to White Creek or the local sewer treatment plant. Long term offsite disposal costs make this option not fiscally desirable.

cc: G. Rider



New York State Department of Environmental Conservation

MEMORANDUM

TO: A. K. Gupta, Operation & Maintenance Section
FROM: John Spellman, Operation & Maintenance Section
SUBJECT: Pollution Abatement Services - Comments on Consultant's Draft Evaluation
DATE: DECEMBER 14, 1990

As you requested, I have read URS Consultant's Draft Report and Interim Plan proposal concerning leachate at the PAS Site.

I concur with URS in that the objective at this site is to contain contamination by keeping a higher head on the exterior side of the slurry wall. To achieve this the leachate needs to be removed from inside the wall and appropriately disposed of.

After debating the advantages and disadvantages of off-site disposal, I would recommend an on-site treatment system. Considering the enormous volume involved, economies-of-scale is going to favor on-site treatment. I would further recommend treated discharge to groundwater as this would raise ^{se-} water levels outside the wall, thus enhancing an inward gradient.

COMMENTS ON TASK 2 AND TASK 3 REPORT:

1. Page 6. How did Severson remove the 391,000 gallons? If they trucked it off-site, was there any opposition to the trucks by local residents? If so, you should reconsider off-site removal. I think a truck route plan with a citizen participation specialist contact will be required for 4 to 5 trucks daily, similar to the Black and Bergholtz Creek project.
2. Page 8. Discharge treated leachate to groundwater if possible.
3. Page 9. "Daily Operator Attention" - I disagree. If on-site treatment is designed right, plant can run continuously at lower gpm (say, less than 40 gpm) without attention for probably two weeks (I'm thinking of the GE sites) or batch at higher gpm with an operator once or twice a week. The holding tank makes it convenient for batch, but with a low-flow continuous mode, pipe size and equipment can be down-scaled.
4. Page 17. What are sun boots?!
5. Appendix B. The force main at the tank is usually below water, at an elevation of 278.5. It looks like the pumping wells are at elevation 260's, so if a stone becomes lodged in the check valves, a flood and serious release could result. I would suggest raising the force main above water level. Heat tracing the exposed ^{should} not be a problem since tracing is proposed for the bubbler. *Pip.ing*
6. Appendix E. A 24,000g tank is a relatively large tank. I question whether this size tank would be necessary for the proposed flow

rate (50 gpm). For comparison, at Love Canal the equalization tank is 6,000g for a 150 gpm flow rate.

7. Air stripper balance. If 20,000 gpd is going into clarifier, then 20,000 gpd is coming out. Also, 84 lb/day of solids is collecting on the carbon bed.
8. Appendix E. Total Suspended Solids appear relatively high for the leachate. ¹⁴⁷⁵ The use of sand filters ~~should be~~ evaluated?
9. Appendix E. An optional flow rate should be determined if on-site treatment is chosen. The reason for choosing 5 gpm and 50 gpm should be qualified. *I realize these are just preliminary calculations*
10. Appendix F. I've read where leachate disposal can cost \$1 to \$2 per gallon.

COMMENTS ON INTERIM PLAN

1. Because the tank volume (24,000g) is the same ^{as} the proposed transfer rate (20,000 gpd), then there can be no down-time on either the pumps or the hauling. *If the pumps fail hauling will be stopped for the same time as the failure period. Similarly if hauling is held up then pumping will have to stop.*
2. The Task 2 and Task 3 report indicated 1 to 3 million gallons needed to be removed before an inward gradient would develop. This plan suggests an inward gradient would develop after 400,000 gallons. I tend to believe the report.

- TOC looks high. If activated carbon is employed, you'll probably ~~not~~ *quite* break through rapidly
- Keep me posted. I'd like to know what course of action is taken. If a moveable treatment system is used let me know - I've got 400,000 g's of leachate elsewhere!

John

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November 30, 1990

Mr. A.K. Gupta, P.E.
Bureau of Western Remedial Action
Division of Hazardous Waste Remediation
NYS Department of Environmental Conservation
50 Wolf Road
Albany, NY 12233-7010

RE: PAS SITE O&M - W.A. D002340-8
Tasks 2 and 3 Report and Revised Analytical Program


Dear Mr. Gupta:

Enclosed please find one copy of the Draft Report for Tasks 2 (Leachate System Evaluation) and 3 (Containment Cell Evaluation) for the above referenced work assignment. Also included is a copy of the Revised Analytical program provided as an addendum to the previously approved Work Plan.

If you have any questions or comments on the enclosed material, please contact me.

Sincerely,

URS CONSULTANTS, INC.

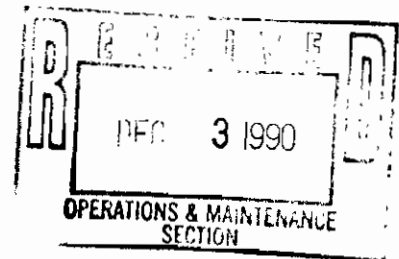

Dharmarajan R. Iyer, Ph.D.
Project Manager

DRI/dri

Enc.

11.30.90.DEC
35236

cc. P. David Smith - NYSDEC (5 copies)
File 35236.00 (1000)





New York State Department of Environmental Conservation

MEMORANDUM

TO: Gerald Rider, O&M Section
FROM: Raymond Lupe, Chief, Central Projects Section, BCRA, DHWR
SUBJECT: Review of URS "Interim Plan to Lower Leachate Levels"

DATE: DEC 21 1990

I have had my staff review the "Interim Plan" submitted by URS. While it is agreed by all that an inward gradient across the slurry wall at PAS is necessary, we strongly feel that the volume of leachate to be removed in the time frame proposed is not feasible.

My staff agrees that the removal of leachate from the containment cell should begin as soon as possible at a realistic rate. This should be done while an on-site treatment system is being implemented. Mr. Edwards agrees with Mr. Gupta's proposal that a removal rate of approximately 120,000 gallons per year should maintain leachate levels within the cell while an on-site system is designed. I feel that regularly scheduled leachate removal and monitoring of the system's response to this removal will provide us with more accurate information than the estimates contained within the "Interim Plan". This approach should not effect the off-site study.

A detailed point by point technical review of the Interim Plan can be supplied by my staff along with our estimates of pumping and recharge rates if you feel it is required at this time. Please keep me advised of what action you intend concerning this "Interim Plan". As always, my staff appreciates the opportunity to work with you in creating an efficient O&M program for the PAS site. If further assistance is needed please contact myself or Bob Edwards at 7-5677.

cc: A. Rockmore
S. Hammond
A. Gupta —
R. Edwards

1/15/91



New York State Department of Environmental Conservation

MEMORANDUM

TO: Distribution List Below
FROM: A.K. Gupta, Environmental Engineer 2, Operation & Maintenance Section
SUBJECT: PAS Site (O&M) Draft Report Task 2 & 3 Appendix D
DATE: DEC 07 1990

AKG

Enclosed please find a darker copy of certain portions (hand written) of Appendix D of the draft report on Task 2 & 3 of Pollution Abatement Services site. These pages were illegible in the draft report sent to you on December 6, 1990. Please replace these pages in your copy of draft report.

If you have any questions, please call me at 457-0927.

Enclosure

DISTRIBUTION:

C. Branagh - Region 7
R. Lupe
P. Waite
J. Strang
J. Spellman
R. Lee

cc: G. Rider

a:pas2&3dr:AKG:et

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December 6, 1990

Mr. A. K. Gupta, P.E., Project Manager
Bureau of Western Remedial Action
Division of Hazardous Waste Remediation
NYS Department of Environmental Conservation
50 Wolf Road
Albany, New York 12233-7010

RE: PAS SITE O&M - W.A. D002340-8
DRAFT REPORT TASK 2 AND 3 - APPENDIX D

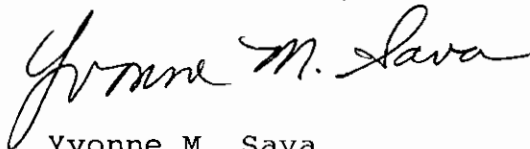
Dear Mr. Gupta:

Pursuant to your conversation today with Dr. Iyer, enclosed please find six (6) sets of darker copies of certain portions (hand written) of Appendix D, per your request.

Please do not hesitate to contact us if we may be of further assistance to you.

Very truly yours,

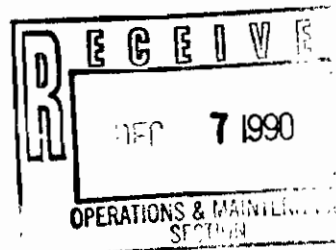
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Yvonne M. Sava
Project Secretary

Enc.

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PAGE II-1 OF
SHEET NO. 1A OF
JOB NO. 35236.00
MADE BY MO DATE 11/26/90
CHKD. BY RJ DATE 11/30/90

PROJECT PAS
SUBJECT Leachate Flow Estimation
IF - Assumptions

THE PURPOSE OF THIS CALCULATION PACKAGE IS TO CALCULATE THE STEADY STATE WITHDRAWAL RATE THAT WOULD ENSURE THAT THE CONTAMINANTS FROM WITHIN THE SLURRY WALL ENCLOSURE DO NOT MIGRATE OFF-SITE. IN ORDER TO DO THIS, SEVERAL ASSUMPTIONS WERE UTILIZED:

1) GEOLOGY

THE GEOLOGY OF THE SITE WAS ASSUMED TO BE AS FOLLOWS: THE UPPER AQUIFER WITHIN THE ENCLOSURE WAS ASSUMED TO HAVE A HYDR. CONDUCTIVITY OF 0.3 FT/DAY. IT'S BOTTOM WAS ASSUMED TO BE AT THE ELEVATION OF 258 FT. THE UNDERLYING LODGMENT TILL WAS ASSUMED TO BE A SEMI-IMPERVIOUS FORMATION WITH THE PERMEABILITY OF 0.03 FT/DAY AND THICKNESS OF 20 FT. THE BEDROCK UNDERNEATH THE TILL WAS ASSUMED TO BE AN AQUIFER
(FOR REF. SEE FINAL REPORT JAN 84, ^{CROSS} SECTION F, P 33)

2) SHAPE

THE SITE IS ROUGHLY 8.5 ACRES. IT WAS ASSUMED, THAT THE SHAPE CAN BE APPROXIMATED BY A RECTANGLE, IT'S WIDTH BEING THE LENGTH OF THE TRENCH CONNECTING WELLS W-1, W-2, W-3 (ABOUT 750 FT). THE LENGTH OF THE RECTANGLE IS

$$L = \frac{A}{\text{WIDTH}} = \frac{8.5 \times 43,560}{750} = 493.7 \text{ FT}$$

SAY 500 FT

SEE FIG ON PAGE 1]

PROJECT PAS
 SUBJECT

REF. PAGE

3) COLLECTION TRENCHES

THERE ARE 2 COLLECTION TRENCHES ON SITE. IT WAS ASSUMED, THAT BOTH OF THEM SPAN THE ENTIRE WIDTH OF THE RECTANGLE THAT APPROXIMATES THE SHAPE OF THE SLURRY WALL ENCLOSURE. THE TRENCH CONNECTING WELLS W-1, W-2, W-3 IS SITUATED NEXT TO THE NORTHERN SLURRY WALL, PARALLEL TO IT. THE TRENCH FOR THE WELL W-4 IS SITUATED IN THE MIDDLE OF THE SITE, PARALLEL TO THE FIRST TRENCH

4) SLURRY WALL

APPROXIMATE FLOW THROUGH THE SLURRY WALL

$$Q = K I A$$

- A - AREA OF SLURRY WALL
- I - GRADIENT ACROSS THE SLURRY WALL
- K - HYDR. COND. OF THE SLURRY WALL

- $K = 1 \times 10^{-7} \text{ CM/S} = 2.8 \times 10^{-4} \text{ FT/DAY (ASSUMED)}$
- $I = \frac{\Delta h}{\text{THICKNESS}}$

$\Delta h_{\text{MAX}} = 271 - 260 = 11 \text{ FT}$ WHERE 271 FT IS THE AVERAGE ELEV. OUTSIDE THE SLURRY WALL AND 260 IS THE LOWEST AVG. ELEV MAINTAINED IN THE COLLECTION DITCHES (ASSUMED, SEE P 10)

THICKNESS = 3 FT

- $A = P \times H$
- P - PERIMETER OF SITE (LENGTH OF SLURRY WALL)
- H - HEIGHT OF SLURRY WALL

PROJECT PAS
 SUBJECT

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PAGE

$$P = 2 \times (750 + 500) = 2500 \text{ FT}$$

$$H = 13 \text{ FT}$$

$$A = 2500 \times 13 = 32,500 \text{ FT}^2$$

$$Q = 2.8 \times 10^{-4} \times \frac{11}{3} \times 32,500$$

$$Q = 33 \text{ FT}^2/\text{DAY}$$

5) CAPPING

A CAPP IS ASSUMED TO BE IMPERVIOUS

6) WATER ELEVATIONS

THE WATER ELEVATION WITHIN THE COLLECTION TRENCH "A" WAS ASSUMED BASED ON THE "ON" AND "OFF" ELEVATIONS FOR PUMPS IN WELLS W-1, W-2, W-3

THE WATER LEVEL IN THE COLLECTION TRENCH "B" WAS ASSUMED BASED ON THE ELEVATIONS OF "ON" AND "OFF" FOR PUMP IN WELL W-4

WELL	"ON" ELEV [FT]	"OFF" ELEV [FT]
W-1	263.7	257.9
W-2	261.6	255.8
W-3	270.7	265.1
W-4	263.0	257.7

SEE FIG ON PAGE 16

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PAGE II-4 OF
 SHEET NO. 10 OF
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PROJECT .. PAS ..
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 PAGE

WATER ELEVATION WITHIN THE BEDROCK AQUIFER WAS ASSUMED BASED ON THE PIEZOMETRIC LEVELS IN NEARBY WELLS (SEE PAGES 14, 15)

SINCE THE LEVELS DIFFER FOR BEDROCK WELLS, IT WAS DECIDED TO PERFORM CALCULATIONS FOR 2 CASES: ~~264~~ 264 FT CORRESPONDING TO WELL LR6 AND 268 FT CORRESPONDING TO WELL LR3

CALCULATIONS WERE PERFORMED FOR 2 SETS OF ELEVATIONS IN TRENCHES AND IN BEDROCK AQUIFER

CASE #	ELEV IN TRENCH "A" [FT]	ELEV IN TRENCH "B" [FT]	ELEV. IN BEDROCK AQUIFER [FT]
1	260	260	264
2	260	262	264
3	260	260	268
4	260	262	268

NOTE: ELEVATIONS IN TRENCHES ASSUMED BASED ON AVERAGES OF "ON" AND "OFF" LEVELS FOR PUMPS (SEE PAGE 16)

PROJECT PAS
 SUBJECT

REF. PAGE

7) APPROACH

THE FOLLOWING APPROACH WAS ASSUMED

- THE WATER LEVEL IN THE COLLECTION TRENCHES WAS ASSUMED CONSTANT
- THE FLOW INTO THE ENCLOSURE WAS ASSUME TO COME FROM THE BEDROCK AQUIFER, THROUGH THE SEMI-IMPERVIOUS LAYER. FOR THAT CASE, A 1-D EQUATION CAN APPROXIMATE THE FLOW (FOR RECHARGE $N=0$)

$$\frac{d^2 h}{dx^2} - \frac{h - \phi_0}{\lambda^2} = 0 \quad \left(\text{HYDR. OF GROUNDWATER, BEAR 1979, P 183} \right)$$

FOR DEFINITION OF PARAMETERS SEE P 3

- THE EQUATION WAS SOLVED USING THE WATER ELEV. IN TRENCHES AND THE LOCATION OF SLURRY WALL AS BOUNDARY CONDITIONS (SLURRY WALL WAS ASSUMED IMPERVIOUS FOR THE PURPOSE)
- THE ASSUMPTION OF THE IMPERVIOUS SLURRY WALL WAS VERIFIED BY COMPARING FLOW THROUGH THE SLURRY WALL FROM PAGE 1C TO THE RESULTING FLOW OBTAINED FROM SOLVING THE EQUATION. IT WAS DETERMINED THAT EVEN THE VERY CONSERVATIVELY EVALUATED FLOW THROUGH THE SLURRY WALL CAN BE NEGLECTED SINCE IT CONSTITUTES LESS THAN 10% OF THE FLOW OBTAINED FROM SOLVING EQ.

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PAGE II-6 OF
 SHEET NO. 1F OF
 JOB NO. 35236.00.
 MADE BY MO DATE 11/26/30
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PROJECT PAS
 SUBJECT Summary of Calculations

8) SUMMARY

REF. PAGE

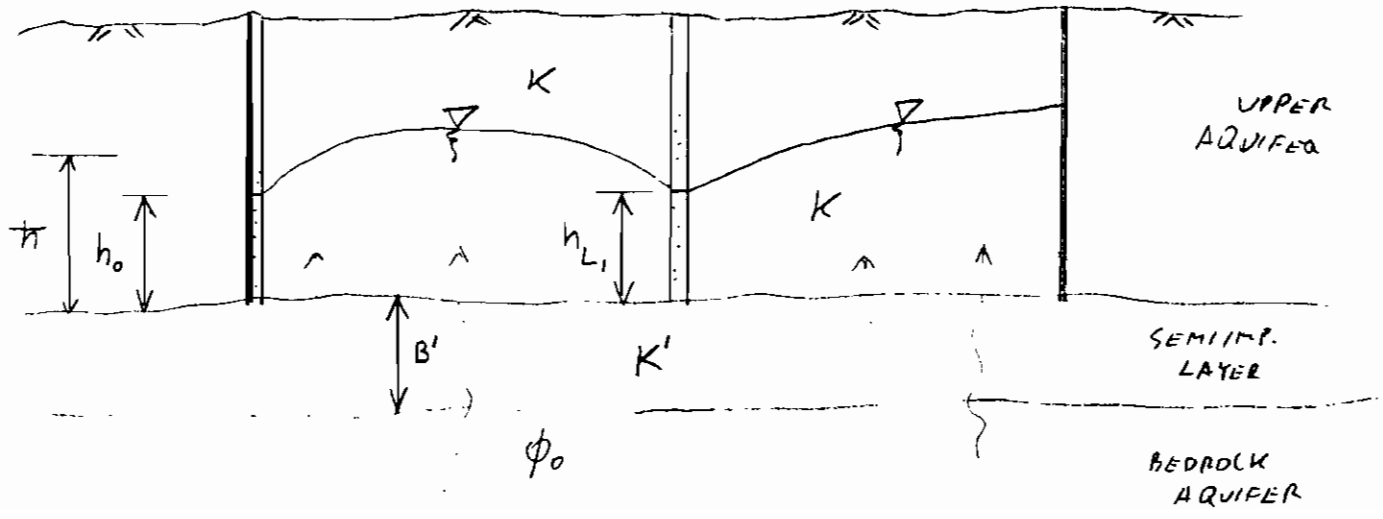
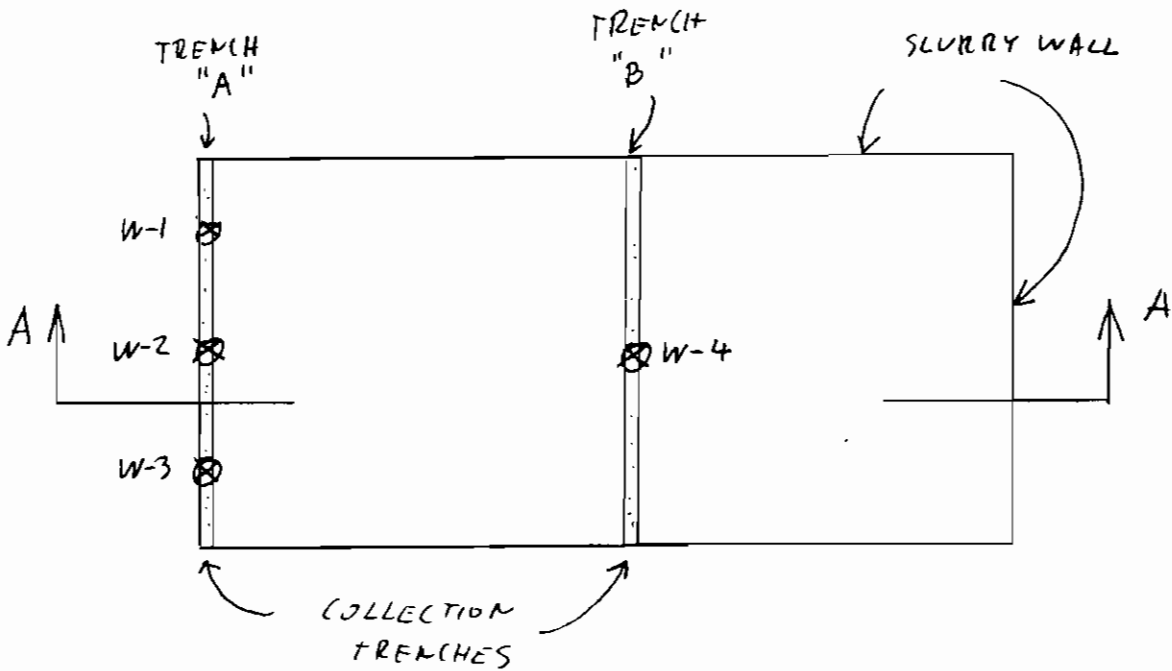
THE FOLLOWING STEADY STATE WITHDRAWAL RATES WERE DETERMINED

CASE #	AVG. WATER ELEV. IN WELLS 1,2,3 [FT]	AVG WATER ELEV IN WELL 4 [FT]	WATER ELEV. IN BEDROCK AQUIFER [FT]	STEADY STATE WITHDRAWAL RATE [GPM]
1	260	260	264	2.0
2	260	262	264	1.4
3	260	260	268	4.9
4	260	262	268	4.2

FOR CALCULATIONS SEE PAGES 2 UP

PROJECT PAS
 SUBJECT Leachate Flow Estimation
 III - Equations & Calculations

REF. PAGE



PROJECT PAS
 SUBJECT

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- ϕ_0 - HYDR HEAD IN THE BEDROCK AQUIFER
- h_0 - HYDR HEAD KEPT IN THE COLLECTION TRENCH FOR WELLS W-1, W-2, W-3
- h_L - HYDR. HEAD KEPT IN THE COLLECTION TRENCH FOR WELL W-4
- K - HYDR CONDUCTIVITY OF THE UPPER AQUIFER
- K' - HYDR. CONDUCTIVITY OF THE SEMIIMPERVIOUS LAYER
- B' - THICKNESS OF THE SEMIIMPERVIOUS LAYER
- L - LENGTH OF THE SITE
- L_1 - HALF OF THE SITE'S LENGTH
- \bar{h} - AVERAGE WATER ELEVATION ACROSS THE SITE
- B_s - WIDTH OF THE SITE

A FLOW WITHIN THE ENCLOSURE CAN BE APPROXIMATED BY A 1-D EQUATION

$$\frac{d^2 h}{dx^2} - \frac{h - \phi_0}{\lambda^2} = 0$$

WHERE : $\lambda^2 = \frac{B'}{K'} K \bar{h}$, $\bar{h} = [\phi_0 + (h_0 + h_L) / 2] / 2$

THE SOLUTION HAS A FORM

$$h = \phi_0 + C_1 e^{-\frac{x}{\lambda}} + C_2 e^{\frac{x}{\lambda}}$$

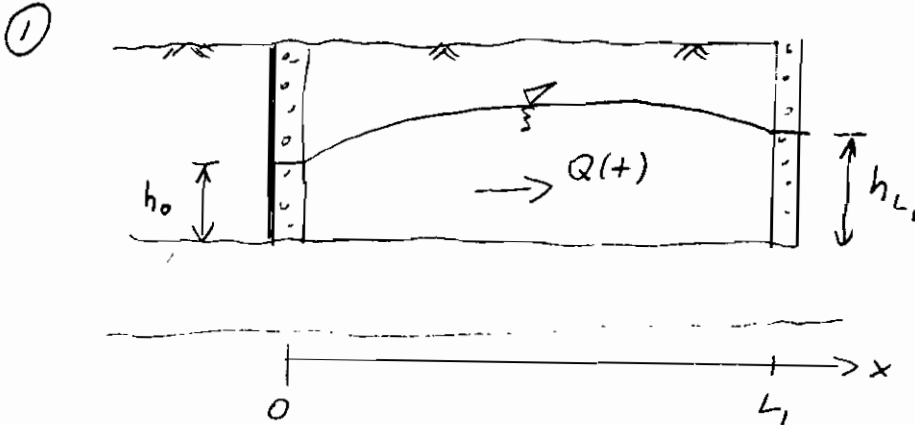
SITE CAN BE DIVIDED INTO 2 REGIONS ?

FROM $x=0$ TO $x=L_1$, AND FROM $x=L_1$ TO $x=L$

PROJECT
SUBJECT

PAS

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PAGE



$$h = \phi_0 + c_1 e^{-\frac{x}{\lambda}} + c_2 e^{\frac{x}{\lambda}}$$

B.C. $h(x=0) = h_0$ (1)
 $h(x=L_1) = h_{L_1}$ (2)

$$\begin{cases} \phi_0 + c_1 + c_2 = h_0 & (1) \\ \phi_0 + c_1 e^{-L_1/\lambda} + c_2 e^{L_1/\lambda} = h_{L_1} & (2) \end{cases}$$

$$\begin{cases} c_1 + c_2 = h_0 - \phi_0 \\ e^{-L_1/\lambda} c_1 + e^{L_1/\lambda} c_2 = h_{L_1} - \phi_0 \end{cases}$$

$$W = \begin{vmatrix} 1 & 1 \\ e^{-L_1/\lambda} & e^{L_1/\lambda} \end{vmatrix} = e^{L_1/\lambda} - e^{-L_1/\lambda}$$

$$W_{c_1} = \begin{vmatrix} h_0 - \phi_0 & 1 \\ h_{L_1} - \phi_0 & e^{L_1/\lambda} \end{vmatrix} = (h_0 - \phi_0) e^{L_1/\lambda} - (h_{L_1} - \phi_0)$$

$$W_{c_2} = \begin{vmatrix} 1 & h_0 - \phi_0 \\ e^{-L_1/\lambda} & h_{L_1} - \phi_0 \end{vmatrix} = (h_{L_1} - \phi_0) - (h_0 - \phi_0) e^{-L_1/\lambda}$$

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 PAGE

$$C_1 = \frac{(h_0 - \phi_0) e^{L_1/\lambda} - (h_{L_1} - \phi_0)}{e^{L_1/\lambda} - e^{-L_1/\lambda}}$$

$$C_2 = \frac{(h_{L_1} - \phi_0) - (h_0 - \phi_0) e^{-L_1/\lambda}}{e^{L_1/\lambda} - e^{-L_1/\lambda}}$$

$$h = \phi_0 + \left[\frac{(h_0 - \phi_0) e^{+L_1/\lambda} - (h_{L_1} - \phi_0)}{e^{L_1/\lambda} - e^{-L_1/\lambda}} \right] e^{-\frac{x}{\lambda}} +$$

$$+ \left[\frac{(h_{L_1} - \phi_0) - (h_0 - \phi_0) e^{-L_1/\lambda}}{e^{L_1/\lambda} - e^{-L_1/\lambda}} \right] e^{\frac{x}{\lambda}}$$

$$Q = -T \frac{dh}{dx} = \frac{T}{\lambda} \left[\frac{(h_0 - \phi_0) e^{+L_1/\lambda} - (h_{L_1} - \phi_0)}{e^{L_1/\lambda} - e^{-L_1/\lambda}} \right] e^{-\frac{x}{\lambda}} +$$

$$- \frac{T}{\lambda} \left[\frac{(h_{L_1} - \phi_0) - (h_0 - \phi_0) e^{-L_1/\lambda}}{e^{L_1/\lambda} - e^{-L_1/\lambda}} \right] e^{\frac{x}{\lambda}}$$

$$Q(A) = \frac{T}{\lambda (e^{L_1/\lambda} - e^{-L_1/\lambda})} \left\{ \left[(h_0 - \phi_0) e^{+L_1/\lambda} - (h_{L_1} - \phi_0) \right] e^{-\frac{x}{\lambda}} + \right.$$

$$\left. - \left[(h_{L_1} - \phi_0) - (h_0 - \phi_0) e^{-L_1/\lambda} \right] e^{\frac{x}{\lambda}} \right\}$$

$$Q_{3-D} = Q \times B_s$$

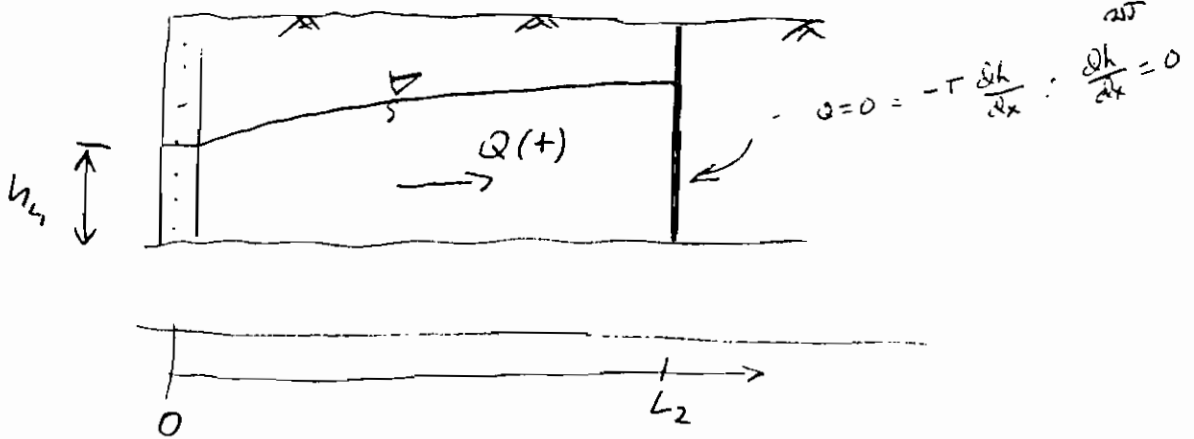
B = WIDTH OF THE SLIT

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 SUBJECT ...

PAS

(2)

REF. PAGE



$$h = \phi_0 + c_1 e^{-\frac{x}{\lambda}} + c_2 e^{\frac{x}{\lambda}}$$

B.C. $h(0) = h_{L1}$ (1)

$$\frac{dh}{dx}(L_2) = 0$$
 (2)

$$\begin{cases} \phi_0 + c_1 + c_2 = h_{L1} & (1) \\ -\frac{c_1}{\lambda} e^{-L_2/\lambda} + \frac{c_2}{\lambda} e^{L_2/\lambda} = 0 & (2) \end{cases}$$

$$\begin{cases} c_1 + c_2 = h_{L1} - \phi_0 \\ -e^{-L_2/\lambda} c_1 + e^{L_2/\lambda} c_2 = 0 \end{cases}$$

$$W = \begin{vmatrix} 1 & 1 \\ -e^{-L_2/\lambda} & e^{L_2/\lambda} \end{vmatrix} = e^{L_2/\lambda} + e^{-L_2/\lambda}$$

$$W_1 = \begin{vmatrix} h_{L1} - \phi_0 & 1 \\ 1 & e^{L_2/\lambda} \end{vmatrix} = (h_{L1} - \phi_0) e^{L_2/\lambda}$$

PROJECT PAS
 SUBJECT

REF.
 PAGE

$$W_{c2} = \begin{vmatrix} 1 & h_{L1} - \phi_0 \\ -e^{-L_2/\lambda} & 0 \end{vmatrix} = (h_{L1} - \phi_0) e^{-L_2/\lambda}$$

$$c_1 = \frac{(h_{L1} - \phi_0) e^{L_2/\lambda}}{e^{L_2/\lambda} + e^{-L_2/\lambda}}, \quad c_2 = \frac{(h_{L1} - \phi_0) e^{-L_2/\lambda}}{e^{L_2/\lambda} + e^{-L_2/\lambda}}$$

$$h = \phi_0 + \frac{(h_{L1} - \phi_0) e^{L_2/\lambda}}{e^{L_2/\lambda} + e^{-L_2/\lambda}} e^{-\frac{x}{\lambda}} + \frac{(h_{L1} - \phi_0) e^{-L_2/\lambda}}{e^{L_2/\lambda} + e^{-L_2/\lambda}} e^{\frac{x}{\lambda}}$$

$$Q(x) = -T \frac{dh}{dx} = -T \left\{ \left[-\frac{1}{\lambda} \frac{(h_{L1} - \phi_0) e^{L_2/\lambda}}{e^{L_2/\lambda} + e^{-L_2/\lambda}} \right] e^{-\frac{x}{\lambda}} + \left[\frac{1}{\lambda} \frac{(h_{L1} - \phi_0) e^{-L_2/\lambda}}{e^{L_2/\lambda} + e^{-L_2/\lambda}} \right] e^{\frac{x}{\lambda}} \right\}$$

$$Q(x) = -\frac{T(h_{L1} - \phi_0)}{\lambda (e^{L_2/\lambda} + e^{-L_2/\lambda})} \left(e^{\frac{x-L_2}{\lambda}} - e^{\frac{L_2-x}{\lambda}} \right)$$

$$Q_{3-D} = Q - B_S$$

B_S - WIDTH OF THE SITE

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ASSUME FOLLOWING VALUES OF THE DATA :

REF. PAGE

$$K = 0.3 \text{ FT/D} \qquad B_s = 750 \text{ FT}$$

$$K' = 0.03 \text{ FT/D}$$

$$B' = 20 \text{ FT}$$

$$L = 500 \text{ FT}$$

$$L_1 = L_2 = L/2 = 250 \text{ FT}$$

CASE 1

$$\phi_0 = 264 - 258 = 6 \text{ FT}$$

$$h_0 = 260 - 258 = 2 \text{ FT}$$

$$h_{L_1} = 260 - 258 = 2 \text{ FT}$$

$$h = (2 + 6) / 2 = 4 \text{ FT}$$

$$\lambda^2 = \frac{20}{0.03} \times 0.3 \times 4 \left[\frac{\text{FT}}{\frac{\text{FT}}{\text{DAY}}} \times \frac{\text{FT}}{\text{DAY}} \times \text{FT} = \text{FT}^2 \right] = 800 \text{ FT}^2$$

$$\lambda = 28.3 \text{ FT}$$

$$T = 0.3 \times 4 \left[\frac{\text{FT}}{\text{DAY}} \times \text{FT} = \frac{\text{FT}^2}{\text{DAY}} \right] = 1.2 \text{ FT}^2/\text{DAY}$$

CASE 2

$$\phi_0 = 264 - 258 = 6 \text{ FT}$$

$$h_0 = 260 - 258 = 2 \text{ FT}$$

$$h_{L_1} = 262 - 258 = 4 \text{ FT}$$

$$h = \left[(2 + 4) / 2 + 6 \right] / 2 = 4.5 \text{ FT}$$

$$T = 0.3 \times 4.5 \left[\frac{\text{FT}}{\text{DAY}} \times \text{FT} = \frac{\text{FT}^2}{\text{DAY}} \right] = 1.35 \text{ FT}^2/\text{DAY}$$

$$\lambda^2 = \frac{20}{0.03} \times 1.35 \left[\frac{\text{FT}}{\frac{\text{FT}}{\text{DAY}}} \times \frac{\text{FT}^2}{\text{DAY}} = \text{FT}^2 \right] = 900 \text{ FT}^2$$

$$\lambda = 30 \text{ FT}$$

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PAGE

CASE 3

$$\phi_0 = 268 - 258 = 10 \text{ FT}$$

$$h_0 = 260 - 258 = 2 \text{ FT}$$

$$h_{L1} = 260 - 258 = 2 \text{ FT}$$

$$t_h = (2 + 10) / 2 = 6 \text{ FT}$$

$$T = 0.3 \times 6 \left[\frac{\text{FT}}{\text{DAY}} + \text{FT} = \frac{\text{FT}^2}{\text{DAY}} \right] = 1.8 \text{ FT}^2/\text{DAY}$$

$$\lambda^2 = \frac{20}{0.03} \times 1.8 \left[\frac{\text{FT}}{\frac{\text{FT}}{\text{DAY}}} + \frac{\text{FT}^2}{\text{DAY}} = \text{FT}^2 \right] = 1200 \text{ FT}^2$$

$$\lambda = 34.6 \text{ FT}$$

CASE 4

$$\phi_0 = 268 - 258 = 10 \text{ FT}$$

$$h_0 = 260 - 258 = 2 \text{ FT}$$

$$h_{L1} = 262 - 258 = 4 \text{ FT}$$

$$t_h = \left[\frac{(2 + 4)}{2} + 10 \right] / 2 = 6.5 \text{ FT}$$

$$T = 0.3 \times 6.5 \left[\frac{\text{FT}}{\text{DAY}} + \text{FT} = \frac{\text{FT}^2}{\text{DAY}} \right] = 1.95 \text{ FT}^2/\text{DAY}$$

$$\lambda^2 = \frac{20}{0.03} \times 1.95 \left[\frac{\text{FT}}{\frac{\text{FT}}{\text{DAY}}} + \frac{\text{FT}^2}{\text{DAY}} = \text{FT}^2 \right] = 1300 \text{ FT}^2$$

$$\lambda = 36.1 \text{ FT}$$



New York State Department of Environmental Conservation

FILE COPY

MEMORANDUM

Originator AKG 12/6/90
Reviewer G. Rider 12/6/90
Reviewer _____

TO: Ray Lupe, Chief, Central Projects Section
FROM: Gerald J. Rider, Jr., Chief, Operation & Maintenance Section
SUBJECT: PAS Site O&M - Interim Plan to Lower Leachate Levels
DATE: 12 06 1990

Enclosed is a copy of URS's submittals of the Interim Plan to lower the leachate levels within the slurry wall and thereby develop an inward gradient all around the contained area.

As you will notice this plan calls for pumping 1 to 3 million gallons of leachate in a short term to bring the leachate to a desirable level. This is a very expensive proposal for an off-site disposal. It is our intent to adjust the schedule for evaluation of on-site treatments. We believe that since the levels have been constant for one year that, at a minimum, pumping should be performed to maintain that level while alternatives are explored and implemented, e.g., on-site treatment. I would appreciate your comments relative to this approach. Will this approach have any adverse effect on the off-site study?

We believe a portable unit or an on-site plant can be made operative within 1 to 2 years, respectively.

I would appreciate your comments and suggestions in this matter by COB December 18, 1990.

Attachment

cc: w/att. - C. Branagh
w/o att. - A. Rockmore

bcc: w/att for comments P. Waite
A. Gupta
J. Strang
J. Spellman
R. Lee

a:pasleach:AKG:GR:et

URS

AN INTERNATIONAL PROFESSIONAL SERVICES ORGANIZATION

URS CONSULTANTS, INC.

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VIRGINIA BEACH
WASHINGTON, D.C.

FAXED November 26, 1990

November 26, 1990

Mr. A. K. Gupta, P.E., Project Manager
Bureau of Western Remedial Action
Division of Hazardous Waste Remediation
NYS Department of Environmental Conservation
50 Wolf Road
Albany, New York 12233-7010

RE: PAS SITE O&M - W.A. D002340-8
INTERIM PLAN TO LOWER LEACHATE LEVEL

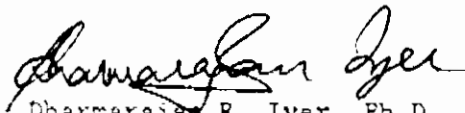
Dear Mr. Gupta:

Enclosed is a slightly revised version of the Interim Plan, which was faxed to you last Wednesday, November 21, to lower the leachate level within the slurry wall and thereby develop an inward gradient all around the contained area. Please review this plan and provide your comments by the end of this week so that we can discuss the next course of action you may wish us to take.

Please also note that we have received only four bids as of today for the hauling and off-site disposal of leachate from the site. Most haulers contacted by us have declined to bid since all of them send their waste to one of the two facilities also bidding on this assignment and feel that they cannot compete with the cost of dealing directly with a disposal facility that can also haul wastes. URS's contract with NYSDEC requires that we obtain five (5) bids for subcontract services. We, therefore, cannot yet select a subcontractor for leachate management unless NYSDEC authorizes us to do so with only four bids. Your assistance in this regard would help expedite this program and meet an already tight schedule.

Sincerely,

URS CONSULTANTS, INC.

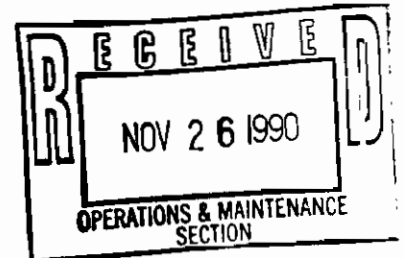


Dharmarajan R. Iyer, Ph.D.
Project Manager

DRI/ys
Enc.

11-26-90.DEC
35236

cc: C. Hurley - URS
File: 35236.00 (1000)



PAS SITE O&M
INTERIM PLAN
TO LOWER LEACHATE LEVEL
WITHIN THE SLURRY WALL

Introduction

URS is currently providing operation and maintenance (O&M) of the Pollution Abatement Services (PAS) site in Oswego, NY through a work assignment under the NYSDEC Standby Contract. This O&M assignment includes an assessment of the leachate collection system, which is underway. However, data obtained over the past year by the NYSDEC from groundwater level measurements in wells on either side of the slurry wall indicate substantially higher water levels inside the landfill compared to levels outside the slurry wall downgradient of the site. Consequently, there is an immediate need to reduce groundwater levels within the slurry wall in order to release the pressure on the containment cell and make the groundwater flow inward at all locations of the slurry wall.

Additionally, a more long term need is to accurately define the amount of groundwater that must be collected and treated from within the slurry wall boundaries to maintain the inward flow gradients to the site once the site has been drained to the desired levels in the inner monitoring wells. An interim plan to accomplish both objectives is presented below.

Rationale

In order to lower the water table within the slurry wall below the levels outside, preliminary calculations indicate that 1 to 3 million gallons of leachate has to be pumped from the leachate collection wells, depending on the desired elevations in the wells. The long term pumping

rate is estimated to range from 2,000 to 7,000 gallons per day (gpd). These estimates are based on limited data available for the site, including hydrogeologic information in previous reports, leachate pumping in 1987 (291,000 gallons) and 1990 (52,000 gallons), and recent groundwater level measurements. The assumptions and calculations used to estimate the leachate withdrawal quantities will be provided in the letter report for Task 2 of this work assignment.

We recommend a phased approach consisting of an initial pumping of leachate, followed by monitoring of the water levels in the wells. This would allow us to immediately lower the water level within the downgradient slurry wall and then re-establish the required pumping rates through data evaluation or a more detailed groundwater flow simulation. The option of off-site disposal versus onsite treatment can then be further evaluated to minimize the total cost of leachate management.

Pumping Program

- o Pump leachate from leachate collection wells LCW-1, 2 and 3 for four weeks at a rate of 20,000 gallons per week day into the leachate holding tank.
- o Remove 20,000 gallons/weekday of leachate offsite for one month. A total of 400,000 gallons will therefore be hauled offsite. Approximately 20% to 30% of this will come from the leachate collection trenches, with the remaining from the formation around the trenches. Within the first few days, there should be a noticeable drop in the wells immediately adjacent to the trench.
- o Monitor all water levels weekly. LCW 4 should be used as an observation well for the first four weeks to determine if the middle of the site is draining. Pumping should be stopped

when inward gradients develop or after four weeks, whichever comes first.

- o After pumping is stopped, continue to monitor water levels in the wells, including the four leachate collection wells for a period of four weeks.

Data from the leachate collection wells can be used to determine the rate at which the site will drain if pumping is continued. This will allow us to fine tune the daily leachate pumping rate necessary to attain the inward gradients. Data from the other wells will be used to determine the impact of the first month's pumping. The estimate for the long-term pumping rate can also be refined with this data.

- o At the end of eight weeks, re-establish leachate pumping but pump from all 4 pumping wells, LCW-1 - 4.
- o Continue monitoring all water levels weekly for 1 year to check for inward gradients during all seasons.
- o Fine-tune the daily leachate pumping to the minimum necessary to just maintain the inward gradients. Vary pumping on a monthly basis at first to allow system to equilibrate.
- o Develop a long-term pumping strategy and compare with onsite treatment options.
- o Contingency - assumes that at end of 2 month initial period, inward gradient have formed, or are very close. Otherwise, 20,000 gpd should be drained until this occurs.



New York State Department of Environmental Conservation

MEMORANDUM

TO: Operation & Maintenance Section Staff
FROM: A.K. Gupta, Environmental Engineer 2, O&M Section
SUBJECT: Pollution Abatement Services (O&M) - Draft Report on Evaluation of Leachate Collection System and Containment Cell Cap. *AKG*
DATE:

DEC 06 1990

Attached is a copy of the above mentioned report received from URS Consultants.

As you will note, the leachate levels within the containment cell are higher than outside groundwater elevations at several locations. Also, included in this report is the estimate of leachate quantity within the containment cell with short-term and long-term leachate flow estimates. The report also includes on-site V/S off-site disposal alternatives. I would appreciate your comments, especially on leachate estimates and disposal alternatives by December 18, 1990.

Attachment

cc: G. Rider

a:pascell.cap:AKG:et



New York State Department of Environmental Conservation

MEMORANDUM

TO: Ray Lupe, Chief, Central Remedial Section
FROM: Gerald J. Rider, Jr., Chief, Operation & Maintenance Section
SUBJECT: PAS Site O&M: WA #D002340-8 Task 2 & 3 Evaluation of Leachate Collection System and Containment Cell Cap
DATE: DEC 06 1990

Enclosed for your information and comments, is a copy of the draft report on the above mentioned subject received from URS Consultants, Inc. If you have any comments please submit them to A. K. Gupta, of my staff, by December 18, 1990.

If you have any questions, please call me or A. K. Gupta at 7-0927.

Enclosure

cc: A. K. Gupta
C. Branagh, w/enc.

a: PAS2&3:AKG:GR:et